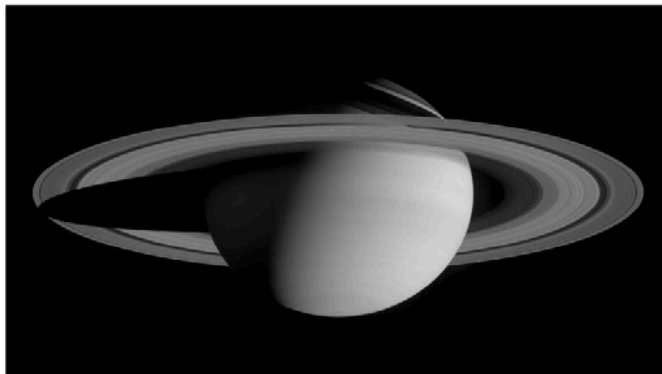
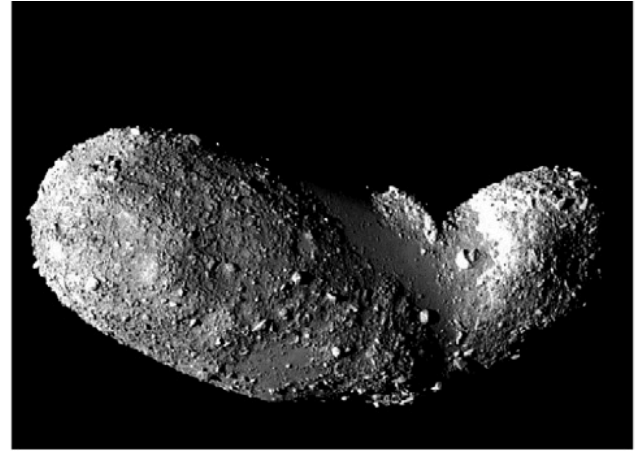
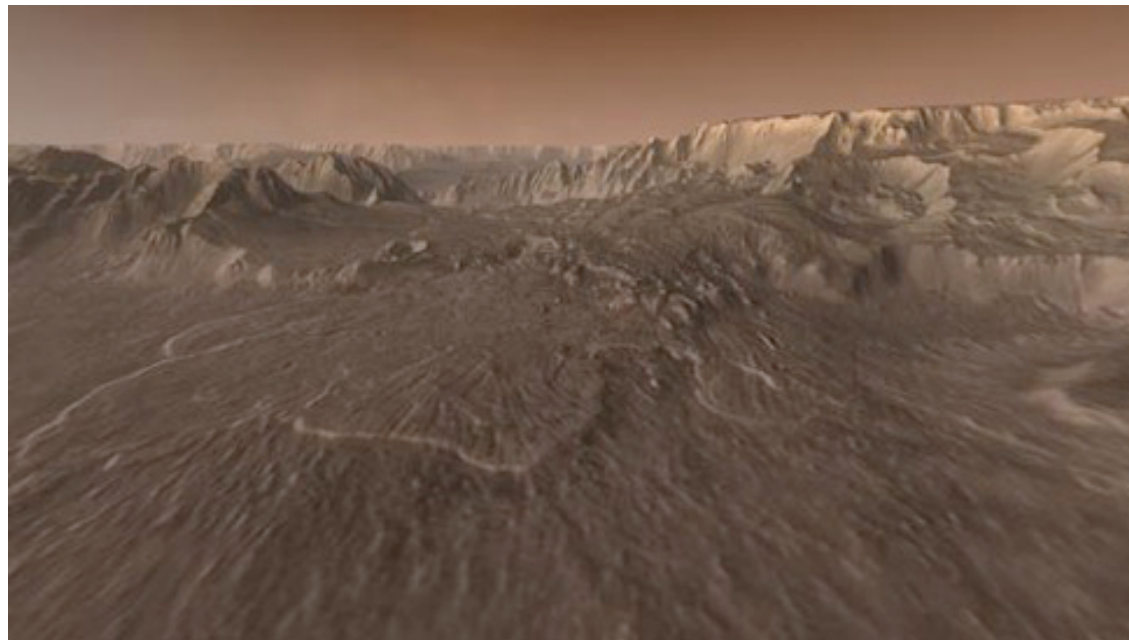
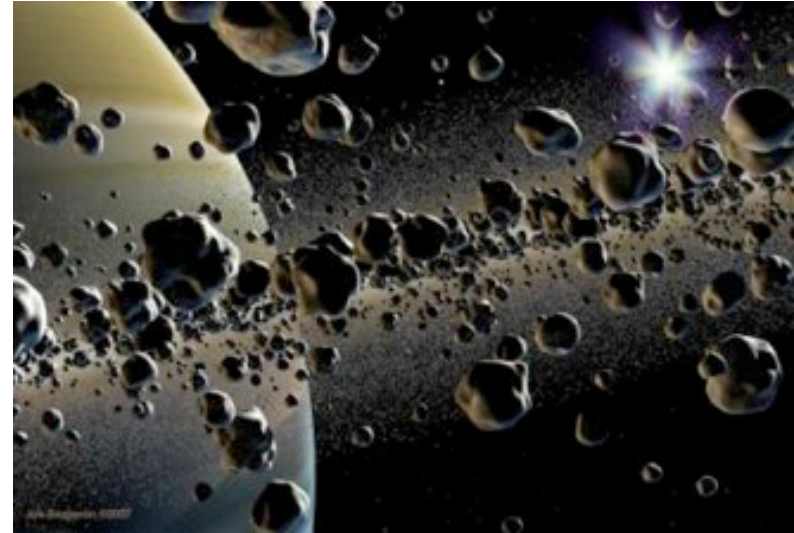


From particles to granular rheology

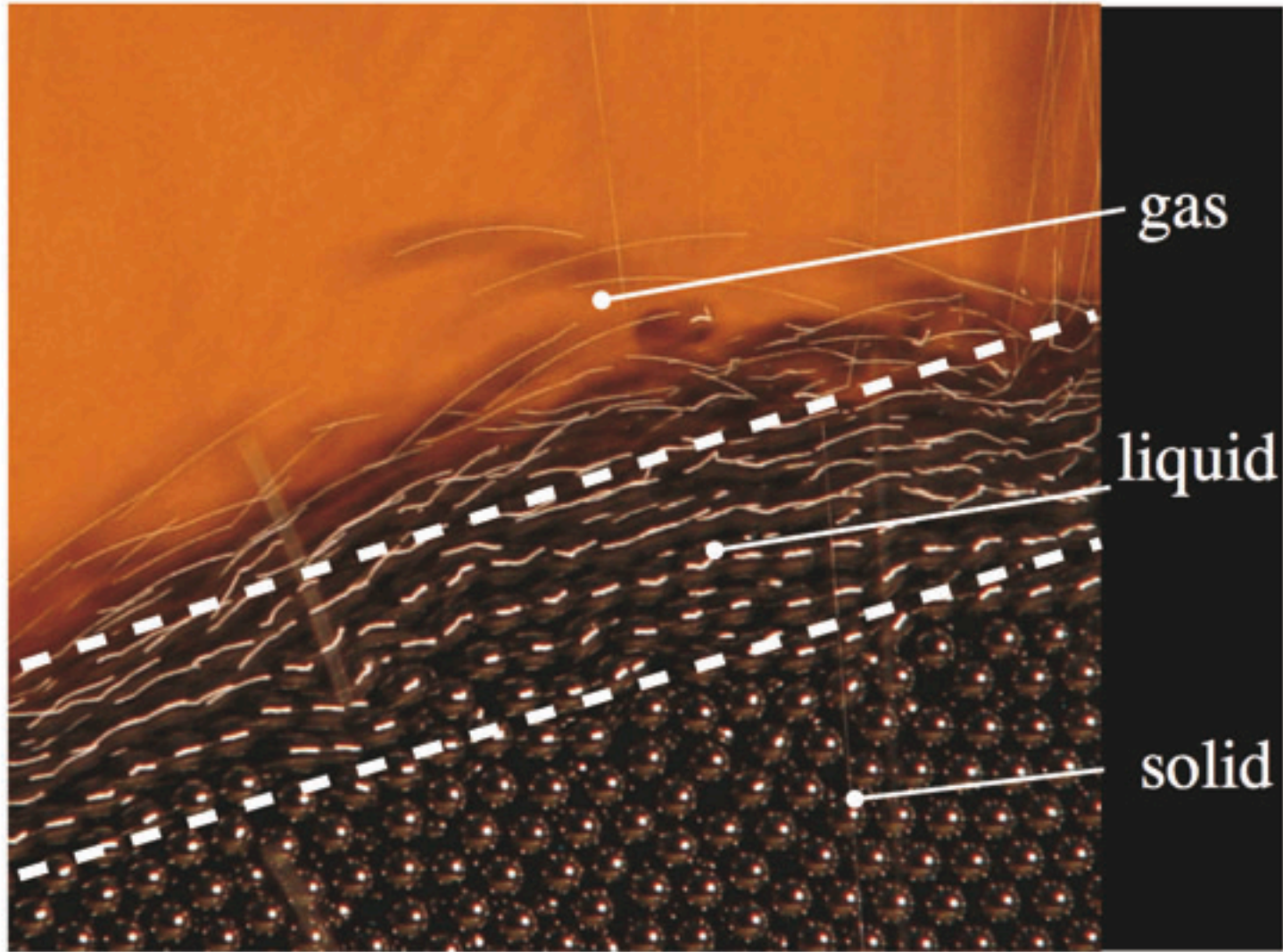
V. Magnanimo
MSM - University of Twente (NL)



Granular material regimes

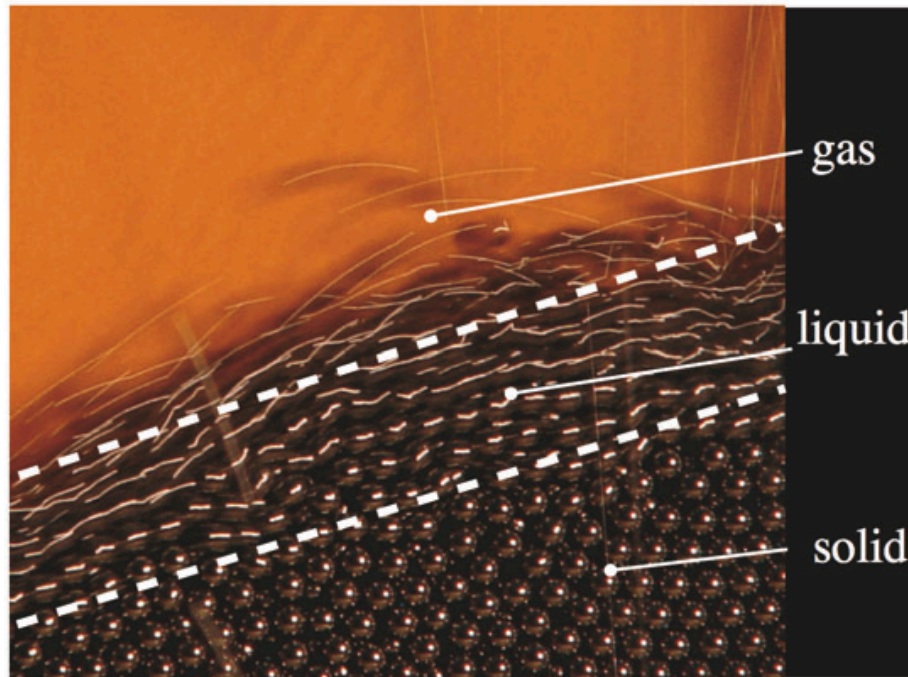


Solid+liquid+gas



[Forterre & Pouliquen, Annu. Rev. Fluid Mech. (2008)]

Solid+liquid+gas



Three regimes:

- **solid** – static
particles interact via frictional contacts
- **liquid** – dense, flow-like behavior
both collisions and friction
- **gas** – rapid dilute flow
particles interact via collisions

Outline

- **Introduction**
- **Internal force transmission**
- **Solid state**
- **Quasistatic regime and flow threshold**
- **Collisional and rapid granular flows**
- **Dense slow flows and inertial regime**

Internal forces transmission

Contact forces

- Unique feature of granular material arises from internal force transmission
- Most fundamental microscopic property of granular materials: irreversible energy dissipation in the course of interaction collision between particles.

Micro-macro transition

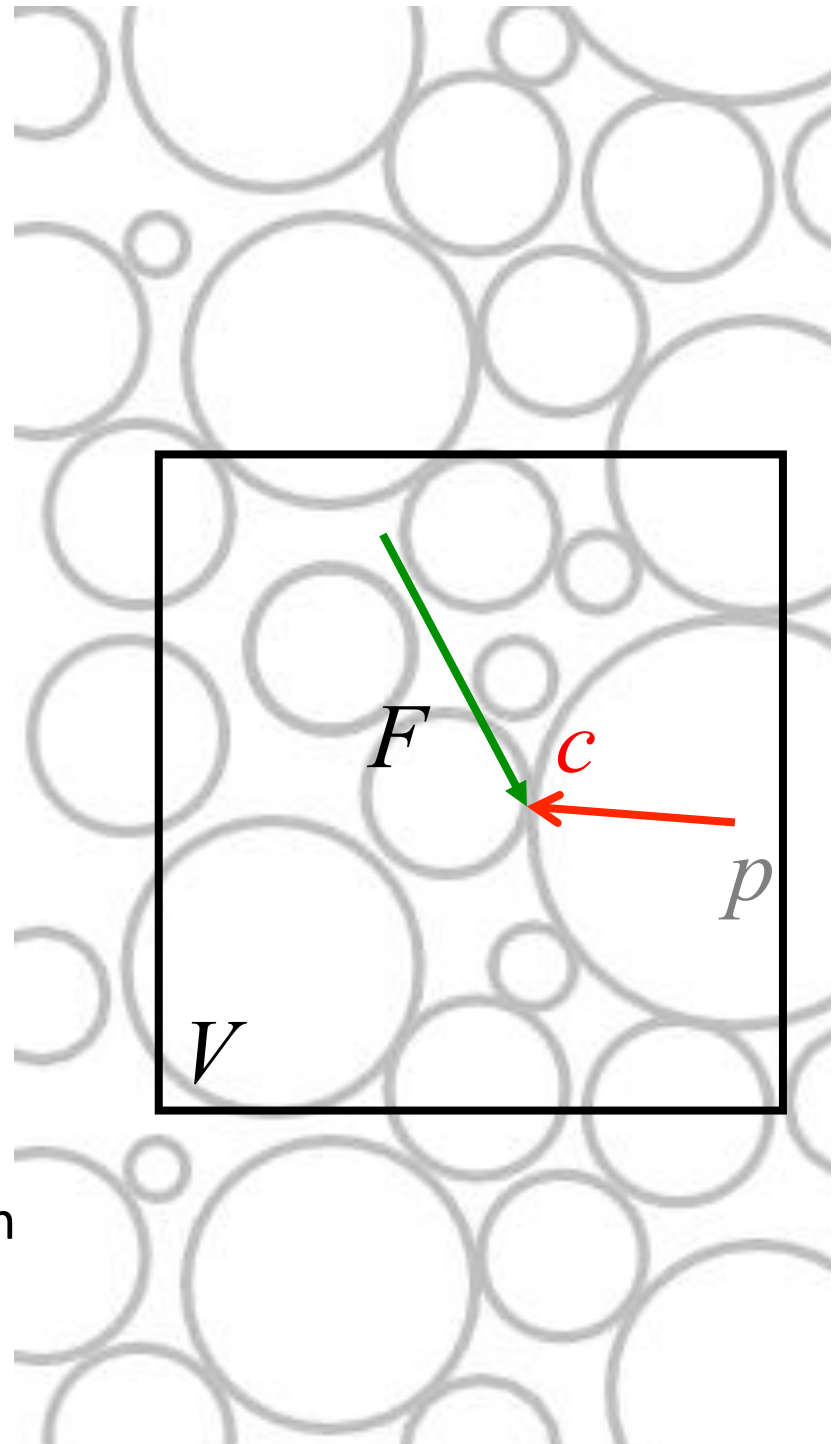
Stress tensor

$$\mathcal{Q} = \frac{1}{V} \sum_c \left(w_V^p \right) \boldsymbol{l}^{pc} \mathbf{F}^c$$

Any quantity:

- Scalar
- Vector
- Tensor: Stress

Overview of more complex formulations in
[Weinhart et al. (2010)]

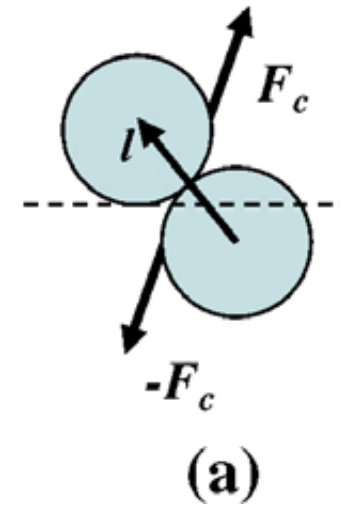


Stress tensor

a. Contact stress tensor

Due to the force transmission across interparticle forces

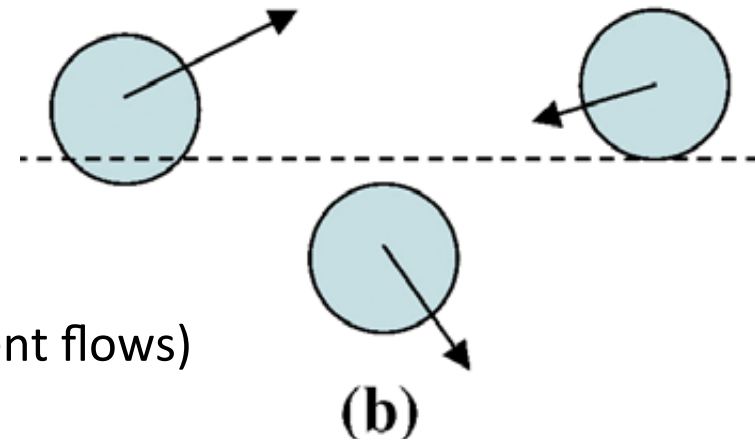
$$\sigma_{ij}^c = \frac{1}{V} \sum_{C=1}^{N_c} F_i^C l_j$$



b. Streaming stress tensor

Due to the motion of a particle relative to the bulk material (Reynolds stress tensor in turbulent flows)

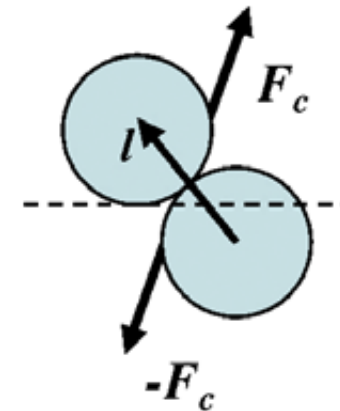
$$\sigma_{ij}^s = \frac{\rho_p \phi}{V} \sum_{p=1}^{N_p} u'_i u'_j$$



Stress tensor

a. Contact stress tensor

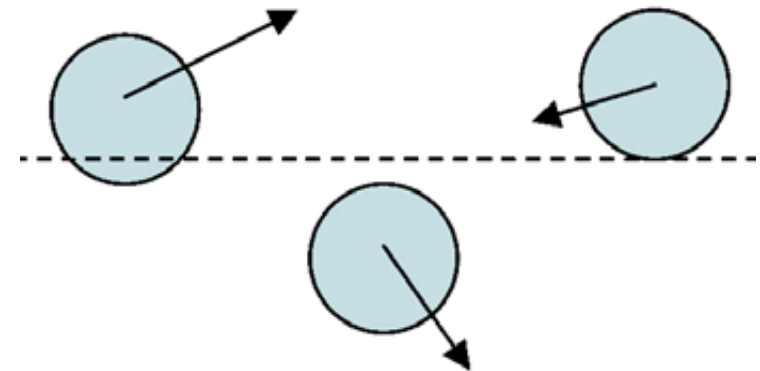
In hoppers, chutes, landslides: $\phi > 50\%$
is usually dominant in common granular flows



(a)

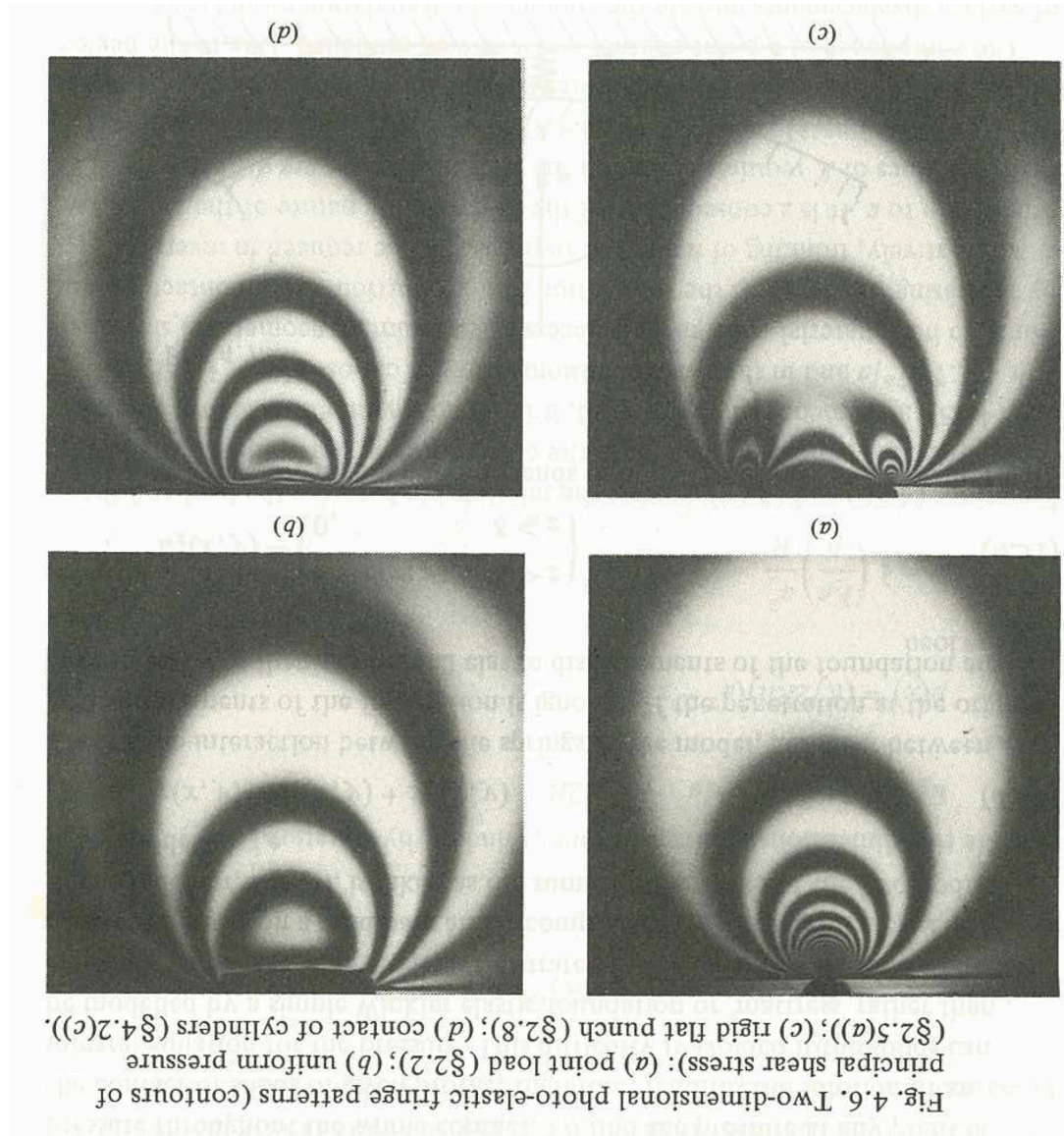
b. Streaming stress tensor

can usually be neglected

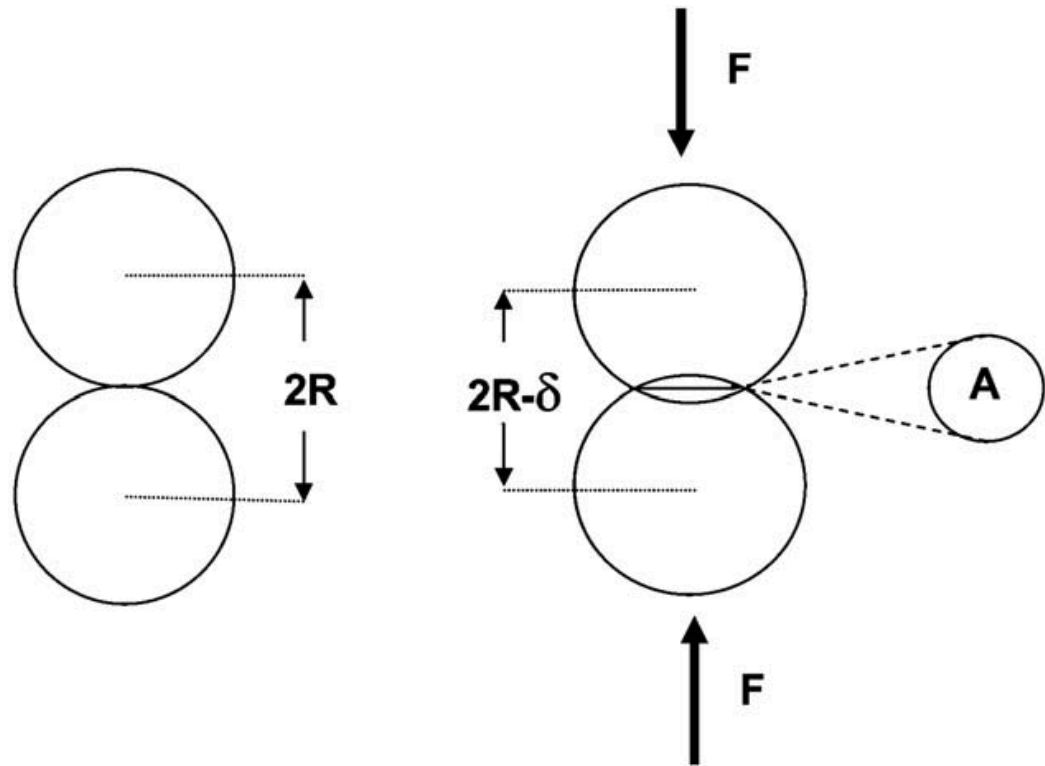


(b)

Hertzian contact law



Hertzian contact law



Contact stiffness

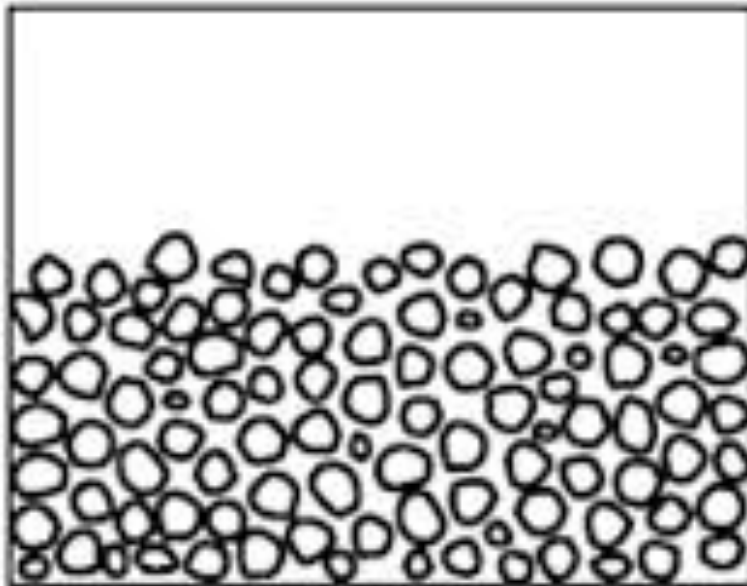
$$k = 6^{1/3} R^{1/3} \left(\frac{E}{1 - \nu^2} \right)^{2/3} F_n^{1/3}$$

Solid state

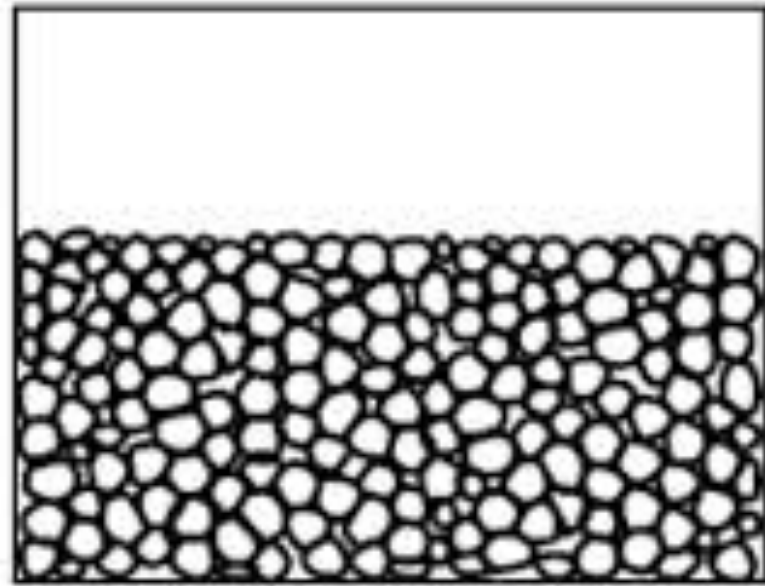
Small strain (elastic) stiffness

Classical solids: elastic stiffness is a material constant

Granular materials: elastic stiffness depends on **pressure** and **volume fraction**

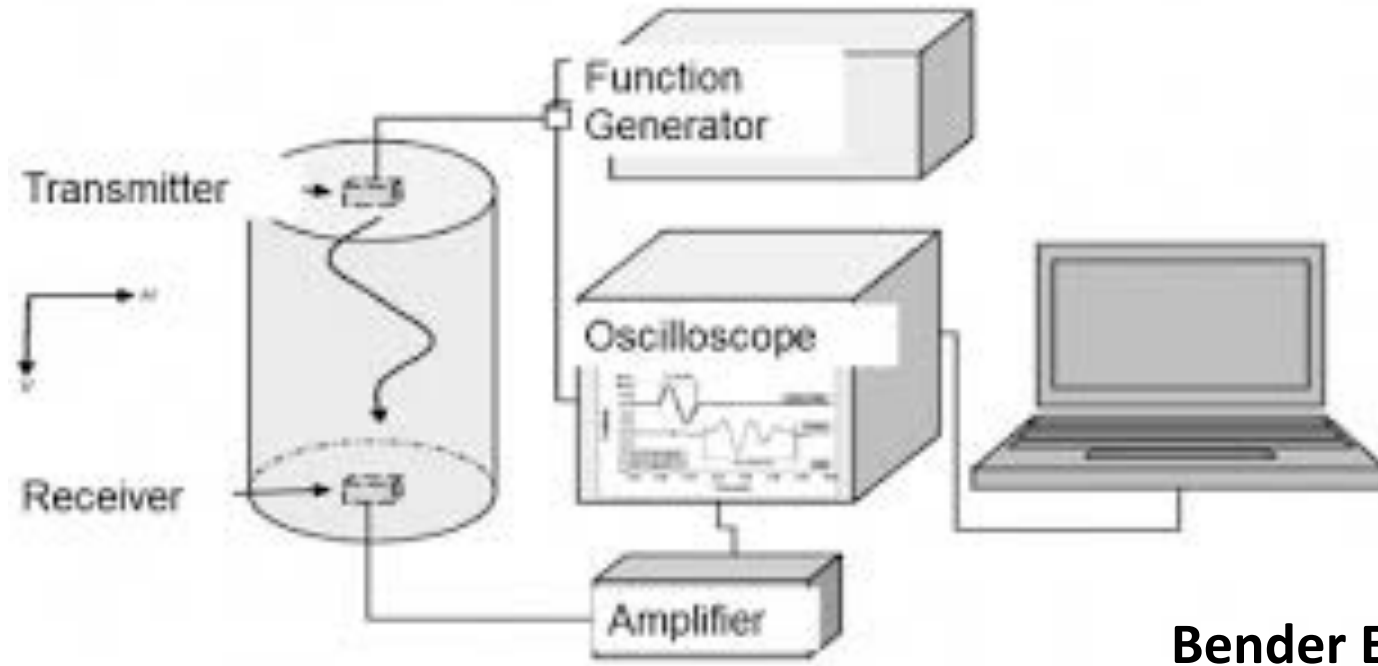


Loose soil (Poor load support)



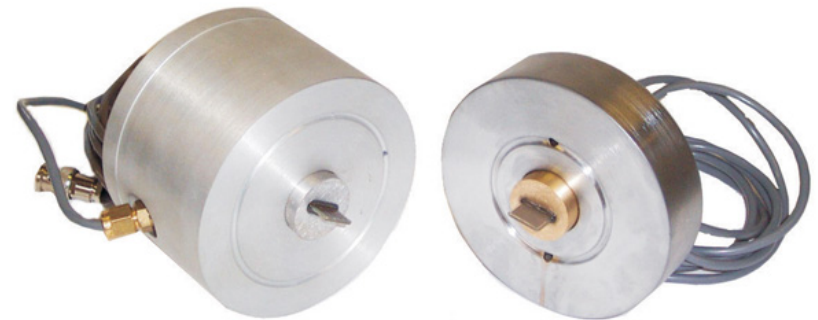
Compacted soil (Good load support)

Small strain (elastic) stiffness

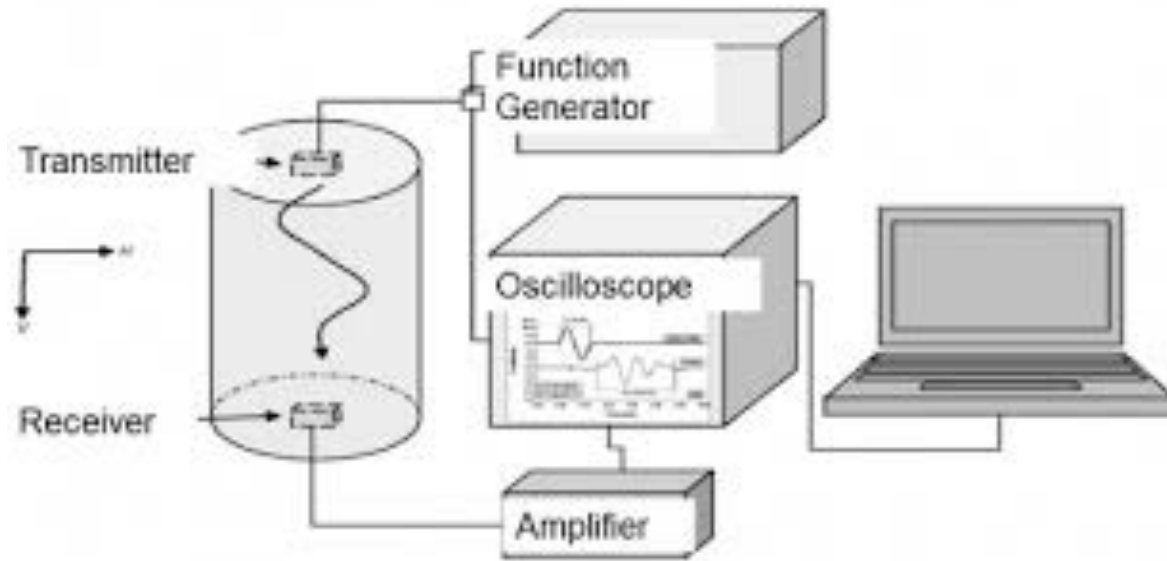


Cell with Bender Elements

Bender Elements



Small strain (elastic) stiffness



Soundspeed

$$V_p = V_p(\phi, \rho)$$
$$V_s = V_s(\phi, \rho)$$

Elastic moduli

$$V_p = \sqrt{\frac{M_{bulk}}{\rho_{bulk}}}$$

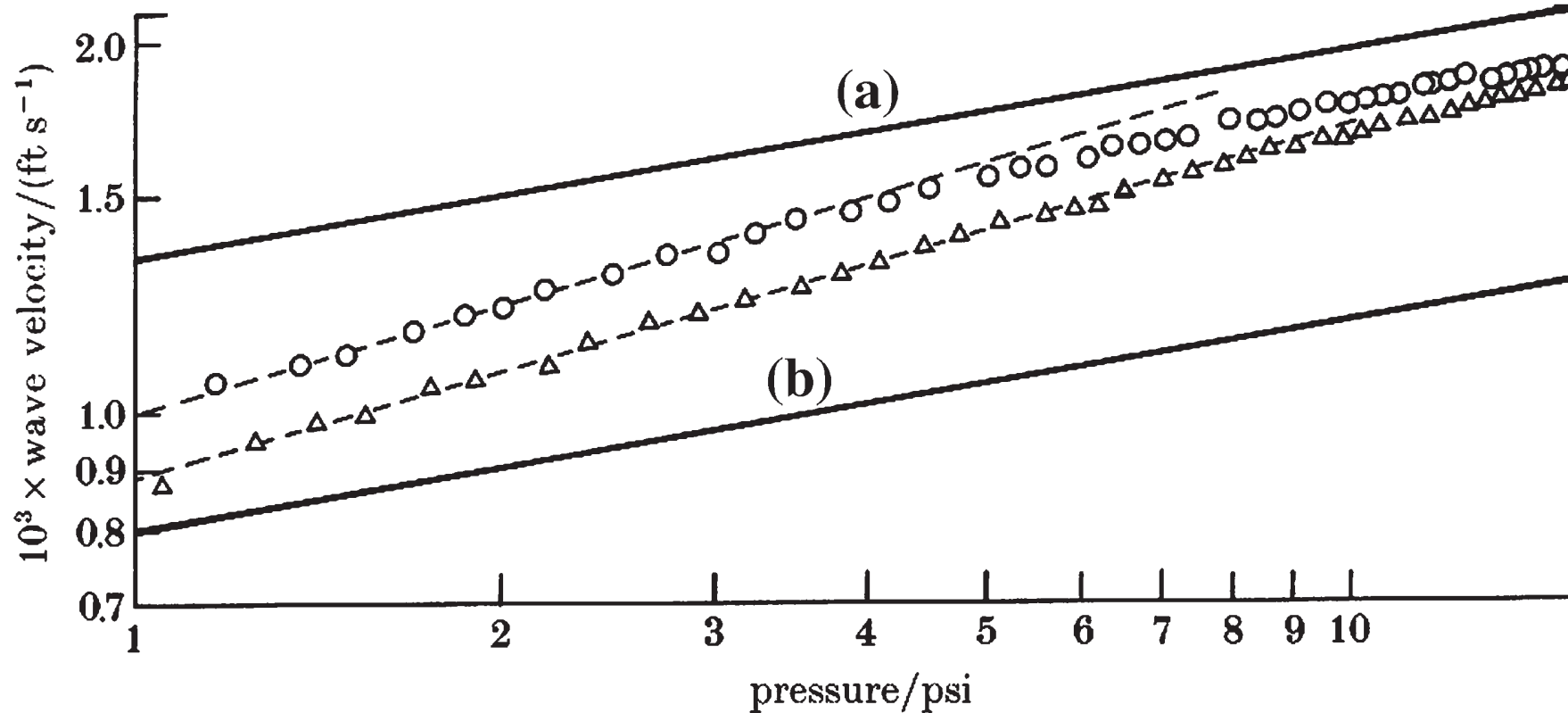
$$V_s = \sqrt{\frac{G_{bulk}}{\rho_{bulk}}}$$

Small strain (elastic) stiffness

Soundspeed

$$V_p = V_p(\nu, p)$$

$$V_s = V_s(\nu, p)$$

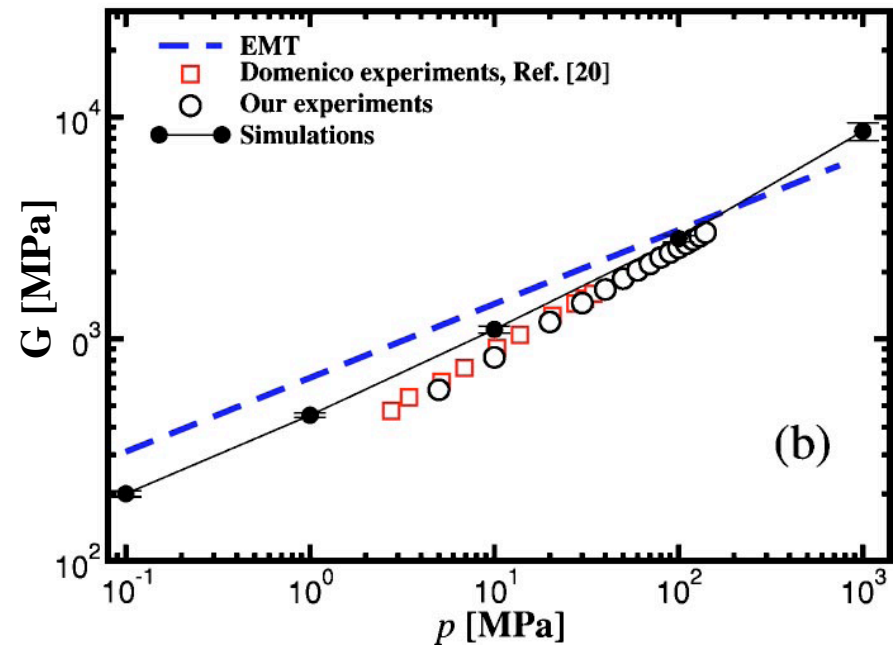
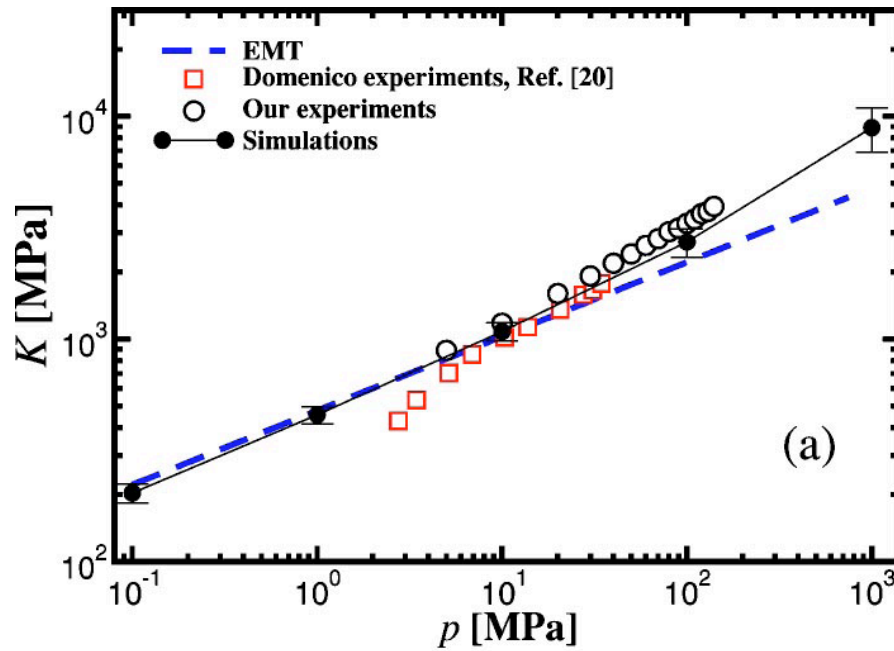


Small strain (elastic) stiffness

$$G_{bulk} \propto \frac{k}{R}$$

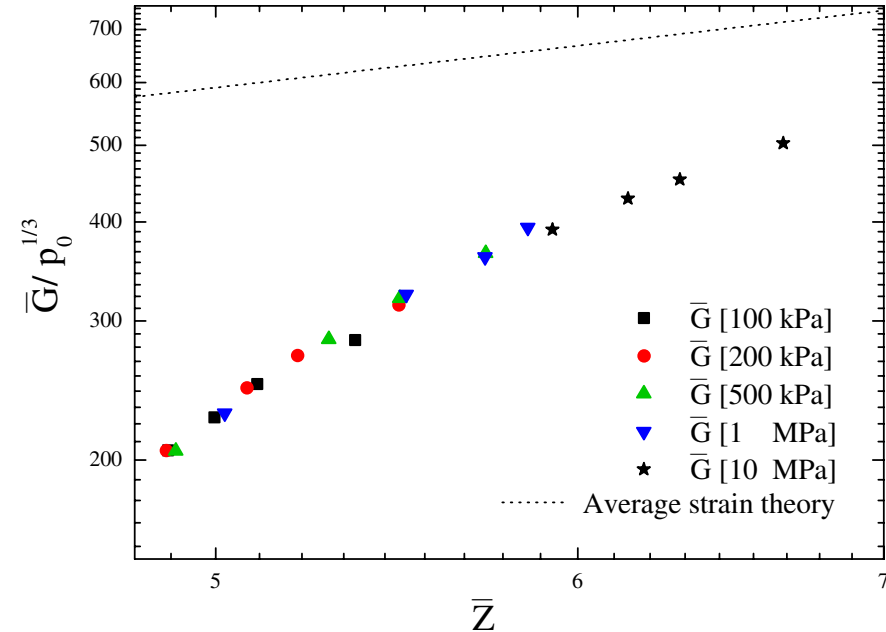
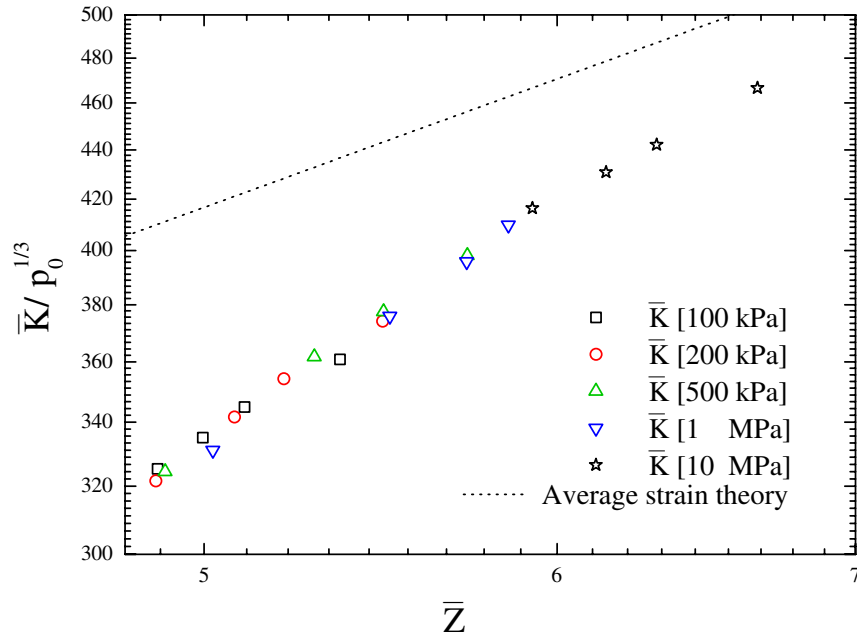
[Bathurst and Rothenburg, J. Appl. Mech. (1988)]

Because of Hertzian interaction we expect: $K(p) \propto G(p) \propto p^{1/3}$



[Gland et al., PRE (2005)]

Dependence on coordination number



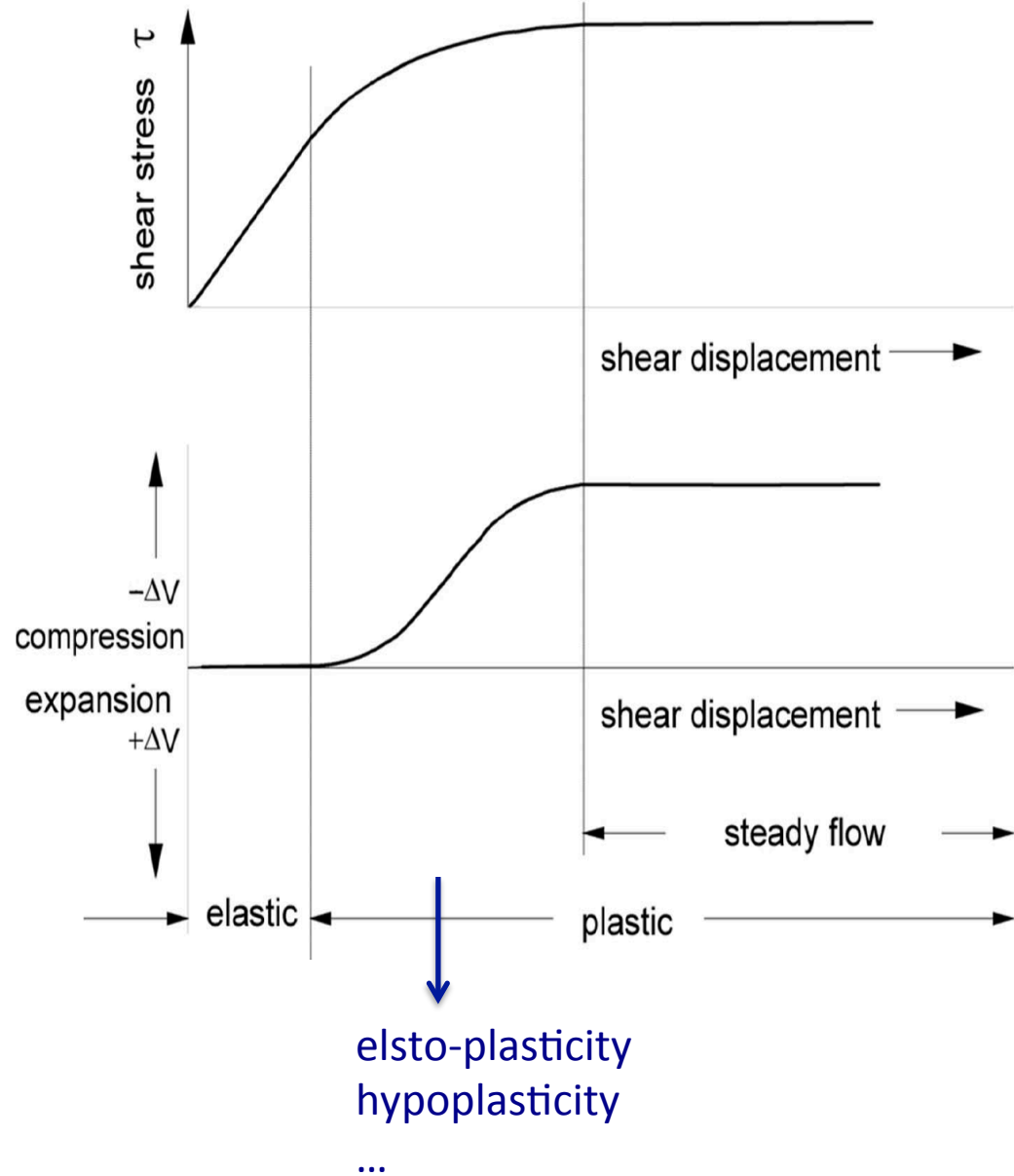
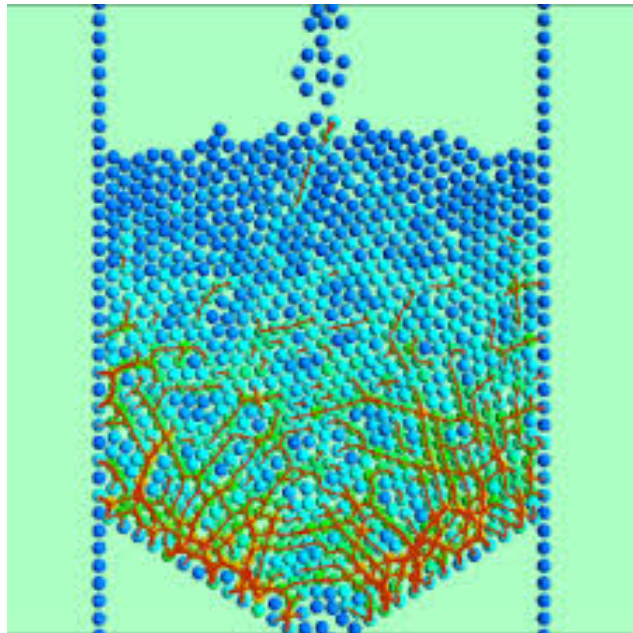
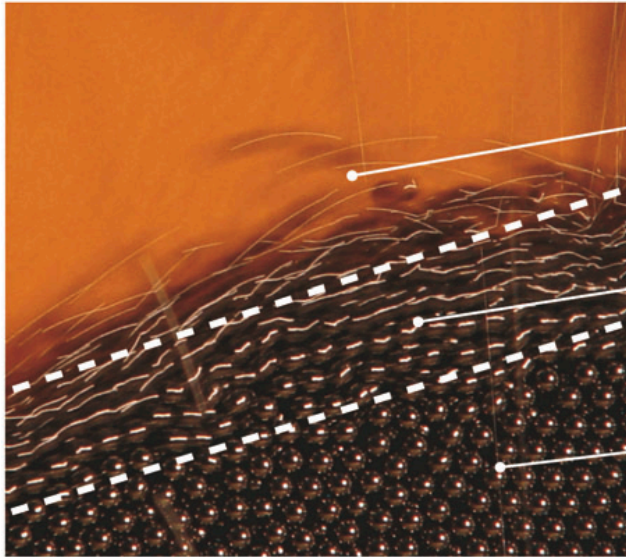
Coordination number

Average number of contacts in the system

$$\bar{Z} = \frac{2Nc}{Np}$$

Quasitatic behavior and flow threshold

Shearing



Quasistatic behavior

Coulomb (1773)

Yielding of granular material as frictional process

Interested in prediction of soil failures for Civil Engineering

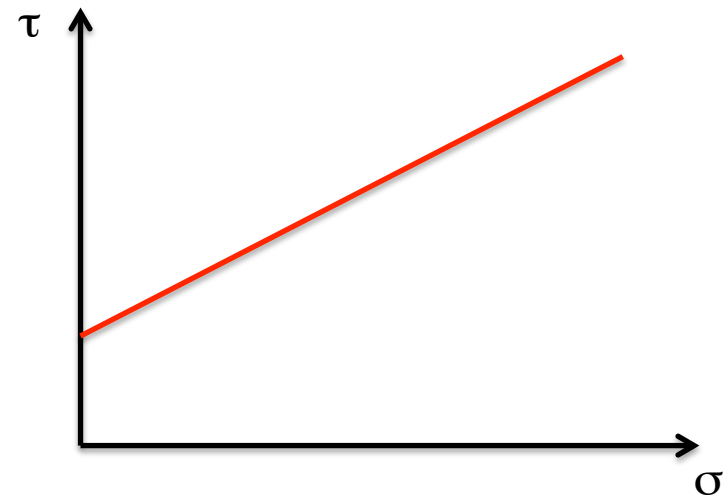
$$\tau < c + \sigma \tan \phi$$

When $\tau = c + \sigma \tan \phi$ the material yields and starts to flow

c = cohesion

ϕ = friction angle

ϕ and c are material constant



Quasistatic behavior

Coulomb (1773)

Yielding of granular material as frictional process

Interested in prediction of soil failures for Civil Engineering

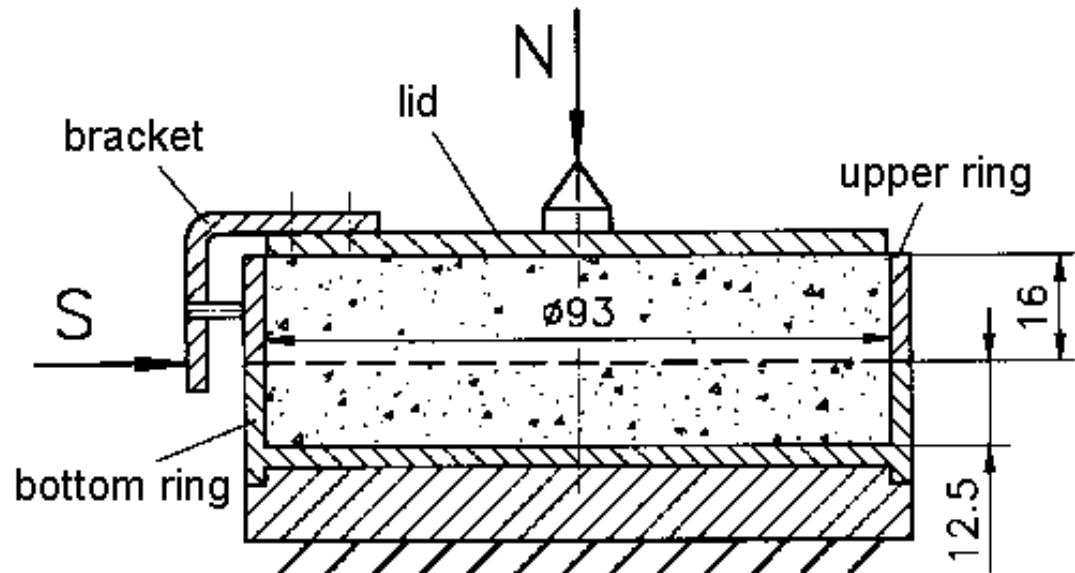
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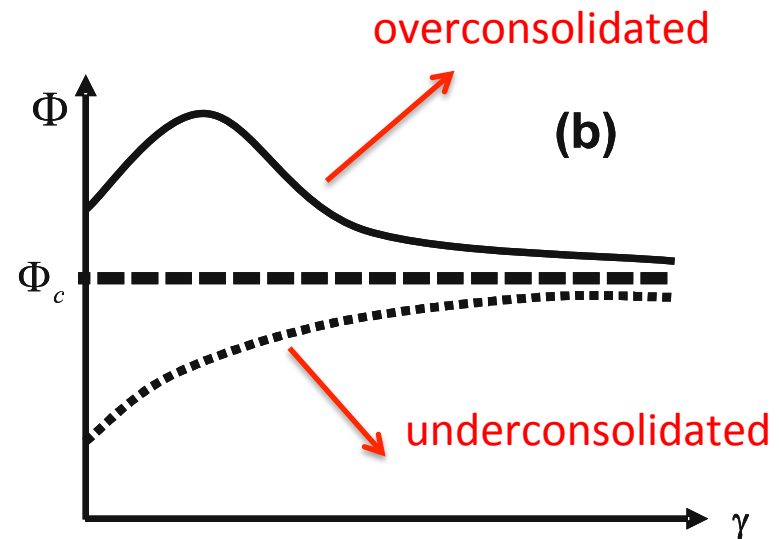
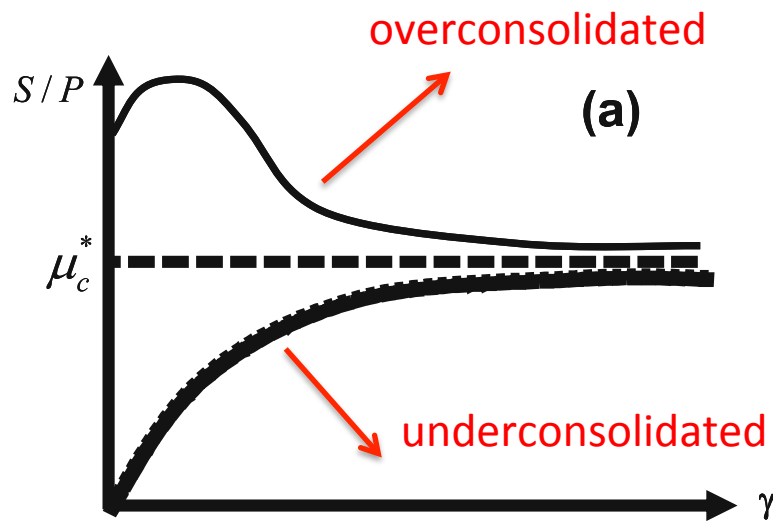
ϕ = friction angle

ϕ and c are material constant



Critical state

A shearing granular material will ALWAYS approach a **critical** concentration
This is the **ONSET OF FLOW**

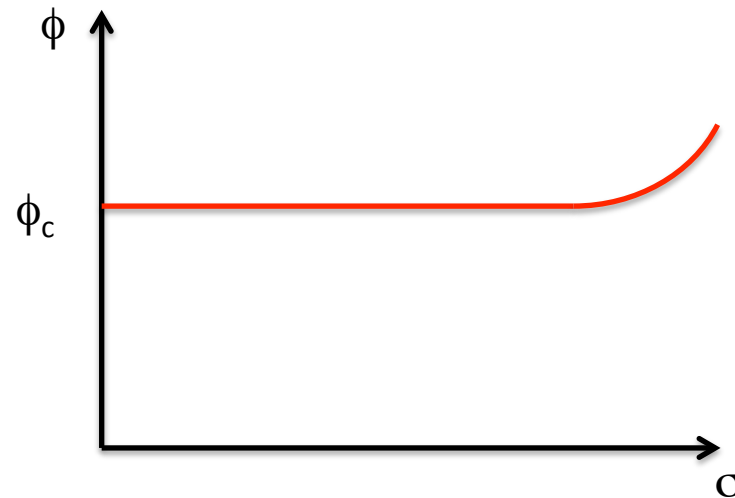


ϕ_c is again a material **constant**

The granular material **DILATES**

Critical state

Variation of critical concentration with applied stress



Most flows are characterized by low stress and large applied strain
→ we can assume incompressible flow at the critical concentration ϕ_c

Critical state

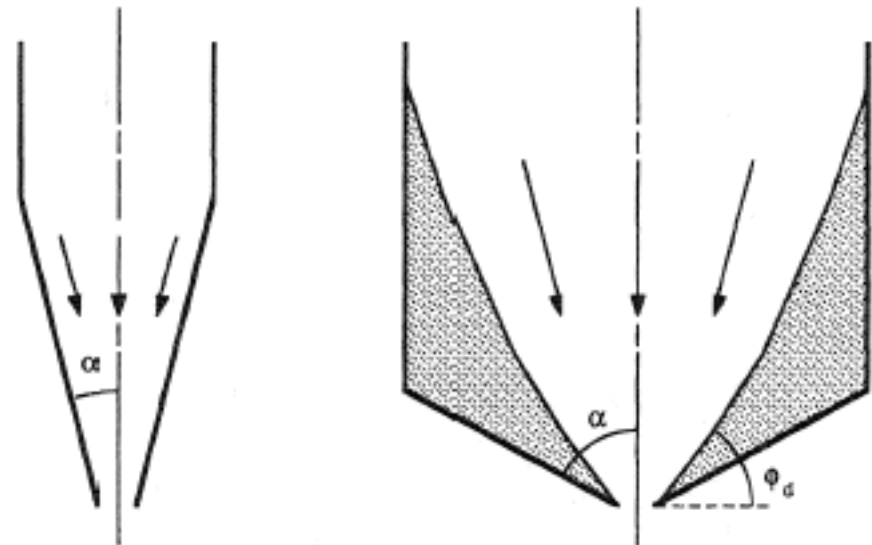
Soil mechanics: widely used

Particle Technology: flow behavior from silo (→ A. Kwade)

- when the material starts flowing is **always** yielding **everywhere** in the hopper (mass flow) or in a region (core flow)

$$\tau = c + \sigma \tan \phi$$

- the material is **always** at the critical concentration and it is **incompressible**.



Critical state

Soil mechanics: widely used

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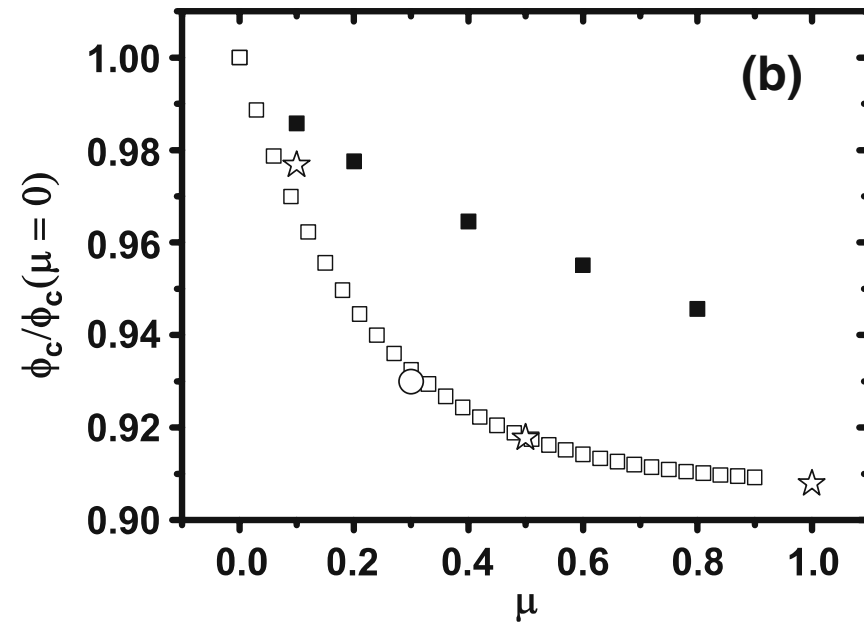
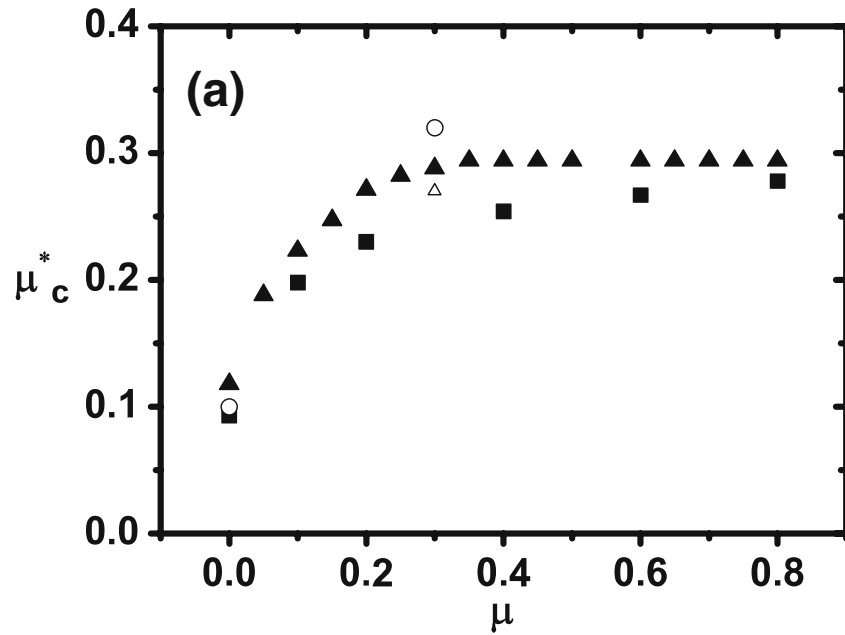
N.B.!!

Application of Critical State theory on is based **on Janssen theory:**

the pressure at bottom of the silo is independent of bed height

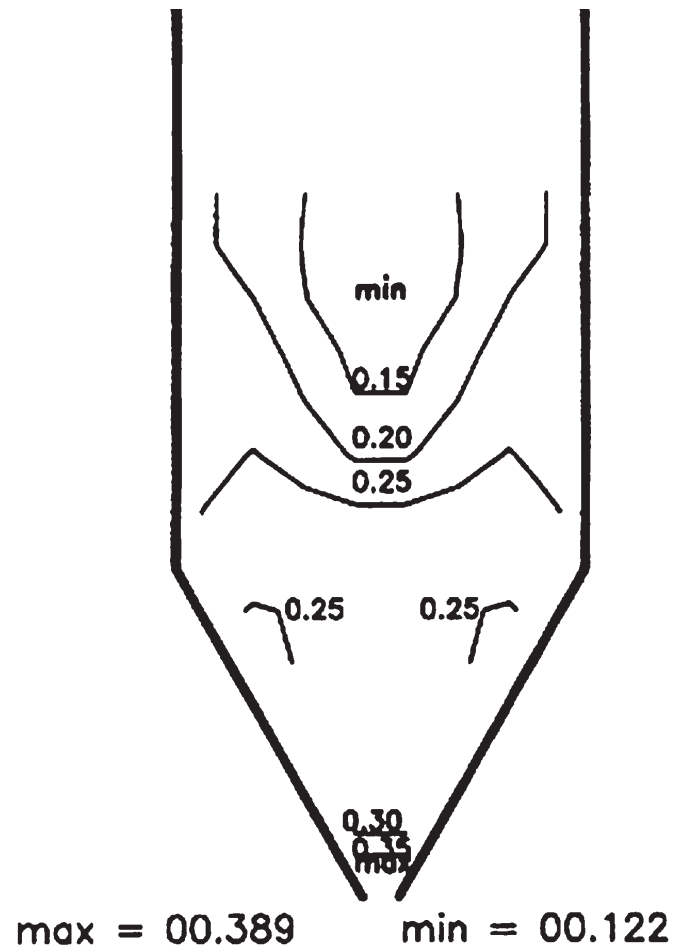
→ the whole bulk material is in the critical state.

Dependence on microscopic properties

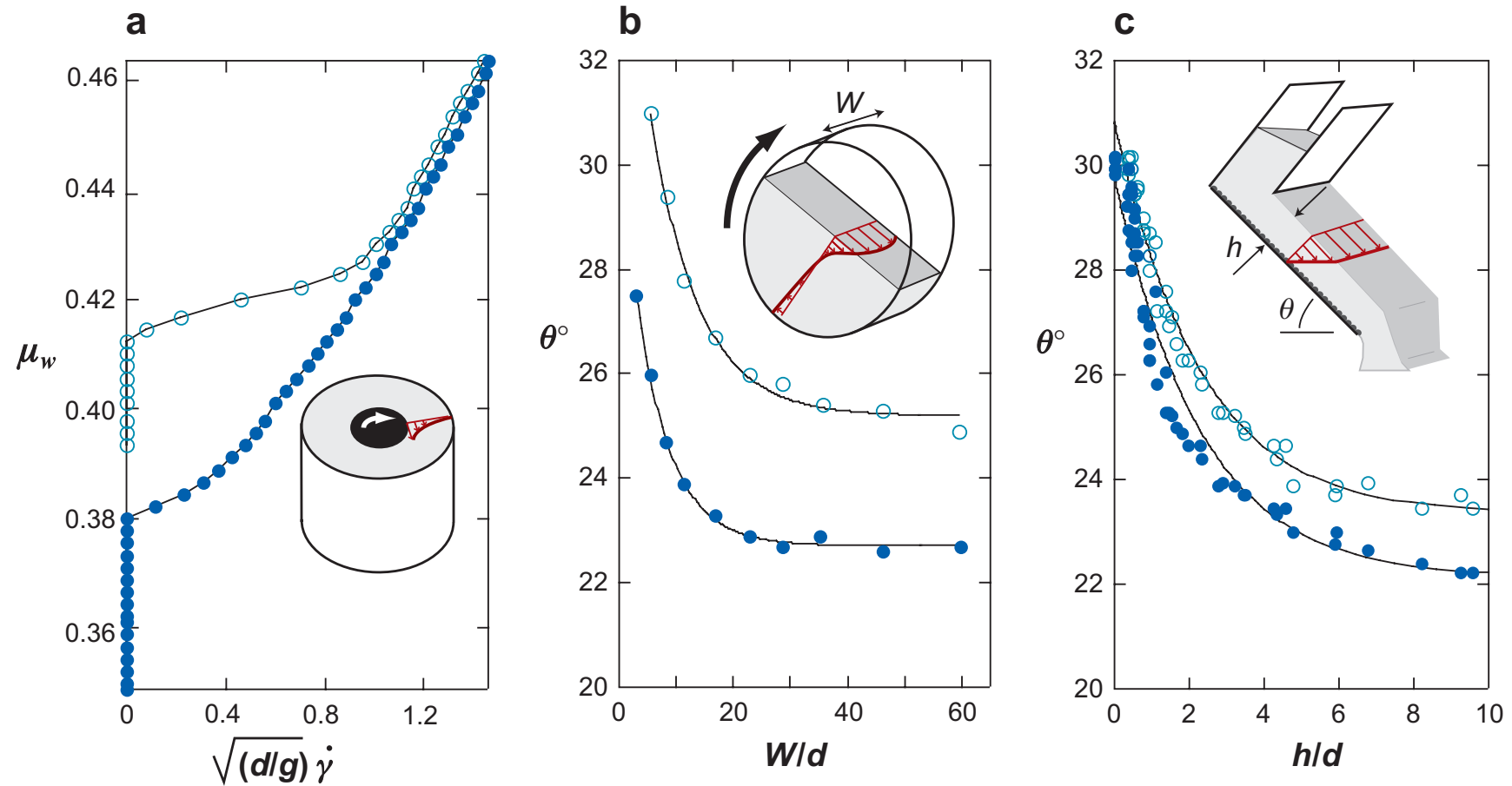


Critical state for silos - problems

ϕ is not constant in the silo

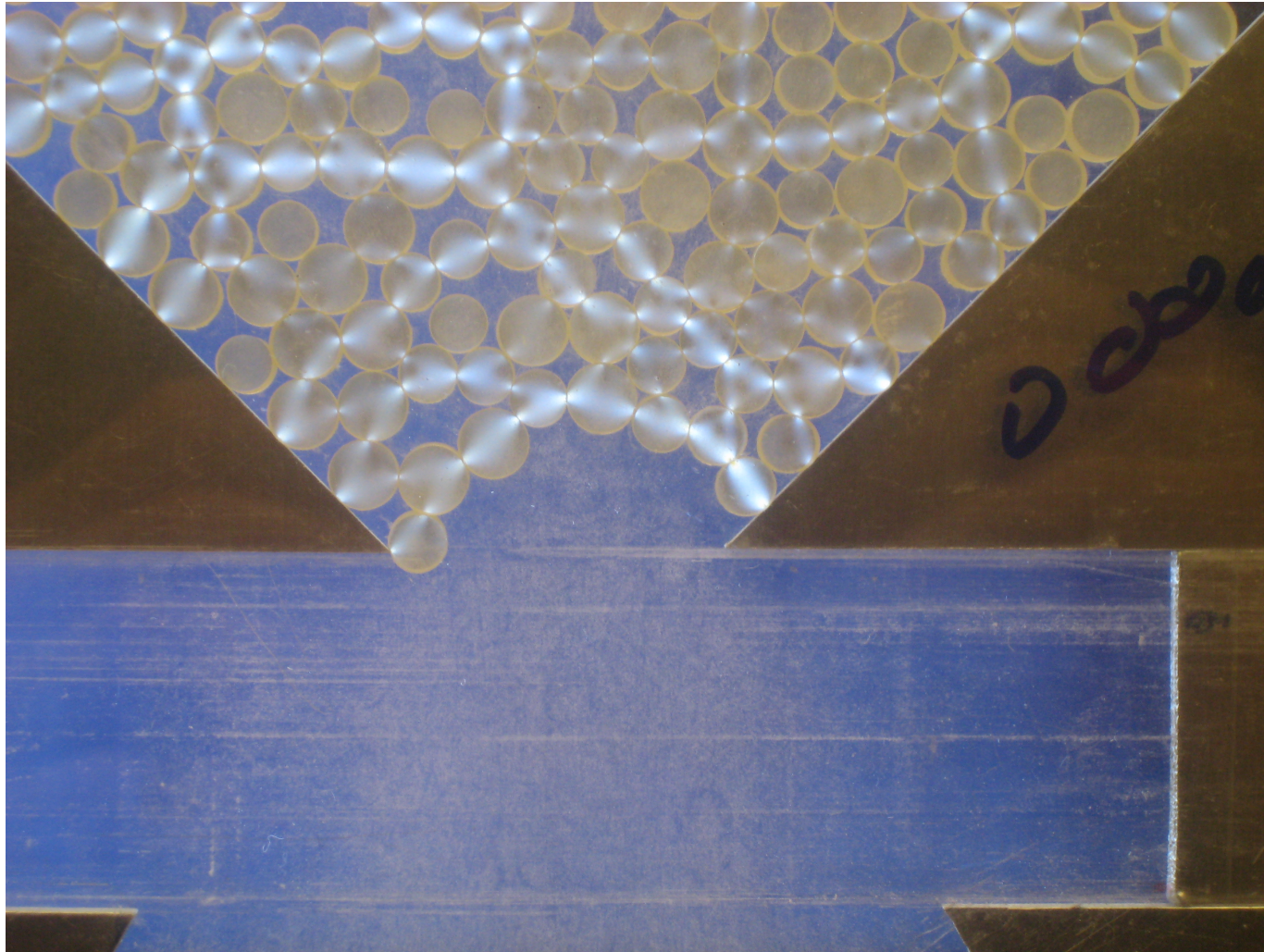


Critical state – can not describe hysteresis



Friction and dilatancy laws

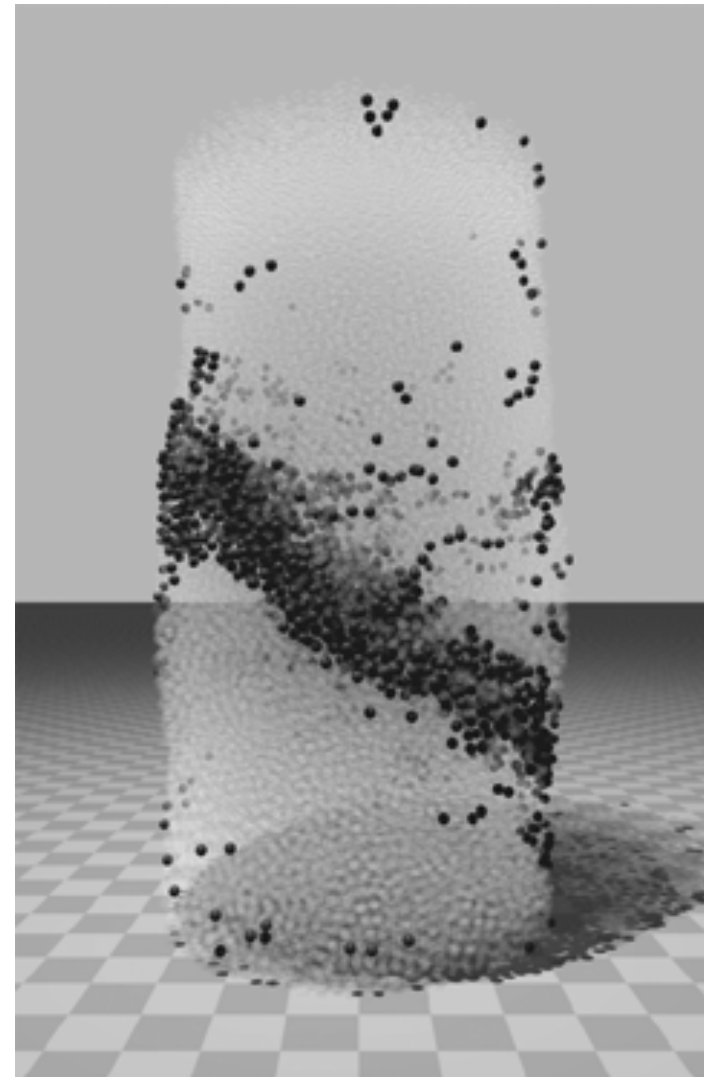
In solid and quasistatic flow, forces are transmitted through **force chains**



Shear bands and dilatant zones

Frictional Behavior = Mohr-Coulomb failure

Granular materials fail along narrow but **finite** zones: **SHEAR BANDS**



Shear bands and dilatant zones

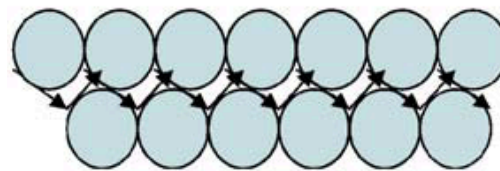
Frictional Behavior = Mohr-Coulomb failure

Granular materials fail along narrow but **finite** zones: **SHEAR BANDS**

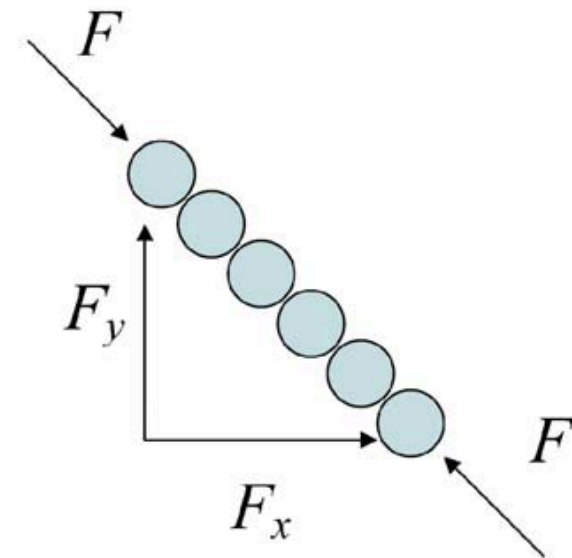
GLOBALLY → frictional behavior

LOCALLY → force chains

$$\frac{\tau_{xy}}{\tau_{yy}} = \frac{\langle F_x l_y \rangle}{\langle F_y l_y \rangle} = \text{const}$$

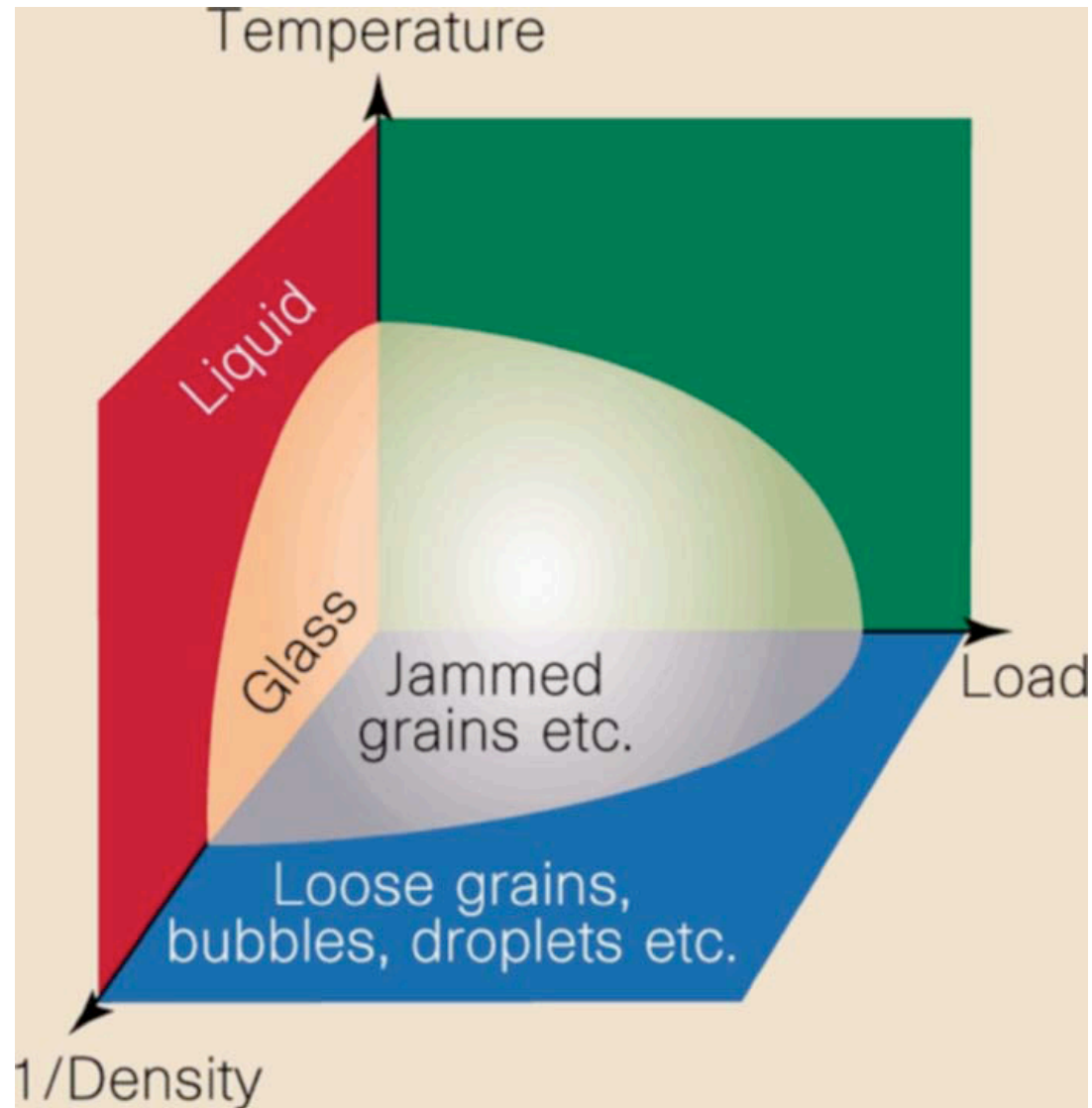


(a)



(b)

Jamming phase diagram



[Liu and Nagel., Nature (1998)]

Jamming robot

Universal Gripper

**U. Chicago, Cornell, iRobot
May 2010**

Collisional or rapid granular flows

Dimensionless analysis (Buckingham Pi theorem)

Bagnold theory

$$\sigma_{ij} = f(\phi, \rho_p, d, \dot{\gamma})$$

$$\sigma_{ij} = f(\phi) \rho_p d^2 \dot{\gamma}^2$$

The shear stress varies as the square of the shear rate

Granular Temperature

1. granules moving in a flow = molecules in the kinetic theory of gases
2. random velocities = thermal motion of molecules.

Granular temperature = magnitude of fluctuating velocities

$$T_g = \frac{1}{3} \left| \langle u_i'^2 \rangle \right| = \frac{1}{3} \left(\langle u'^2 \rangle + \langle v'^2 \rangle + \langle w'^2 \rangle \right)$$

Trace of the streaming stress tensor

$$T_g = \frac{1}{3\rho\phi} tr(\sigma_{ij}^s)$$

deriving a set of equations for **Rapid Granular Flows**

Granular Hydrodynamic

Conservation of mass

$$\frac{D\rho\phi}{Dt} = \rho\phi\nabla \cdot \underline{u} = \mathbf{0}$$


Conservation of momentum

$$\rho\phi \frac{D\underline{u}}{Dt} = \nabla p(\rho, \phi, T_g, e) + \nabla \cdot (\eta(\rho, \phi, T_g, e) \nabla \underline{u})$$

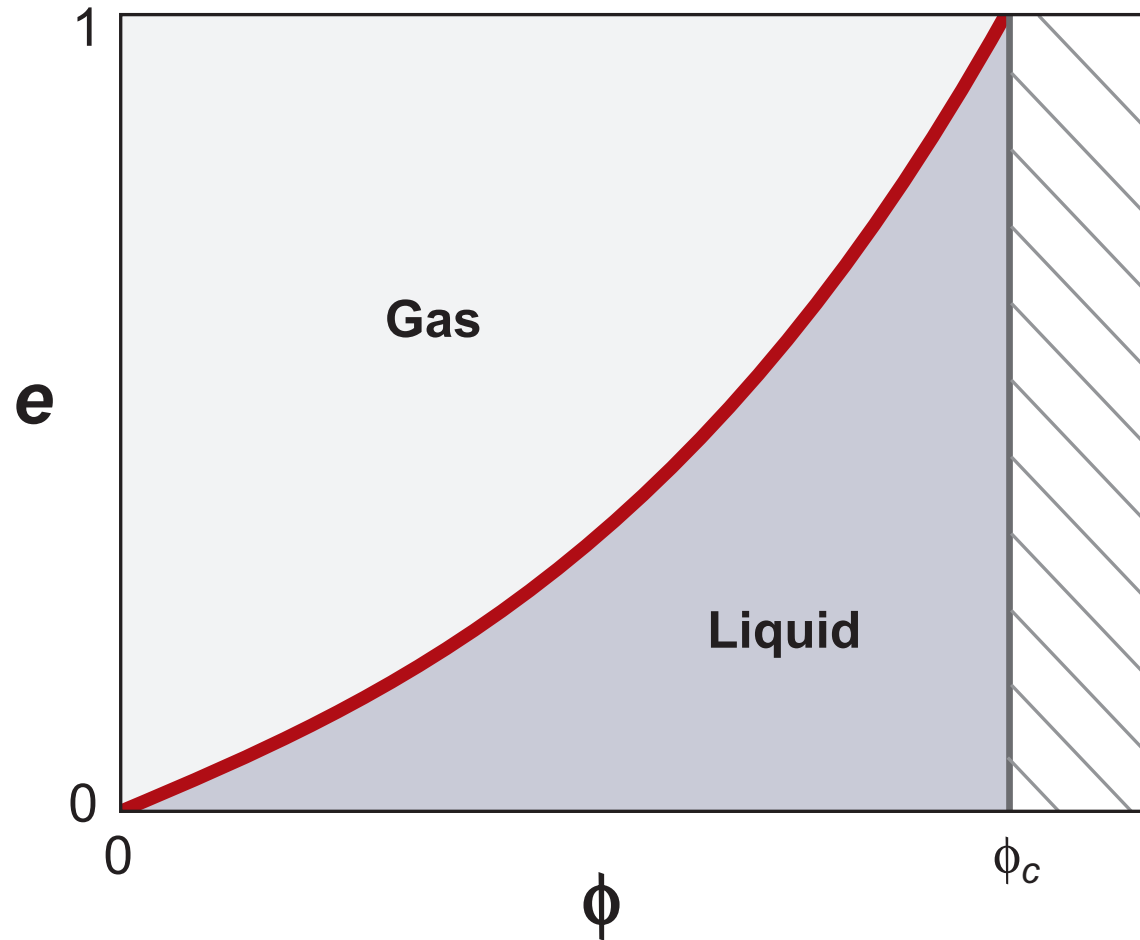
Conservation of granular energy (granular temperature)

$$\rho\phi \frac{DT_g}{Dt} = \nabla \cdot (\alpha(\rho, \phi, T_g, e) \nabla T_g) + \underline{\sigma} \cdot \nabla \underline{u} - \Gamma(\rho, \phi, T_g, e)$$

Kinetic Theory – Range of applicability

- **Nearly elastic particles** ($e=0.9$)
 - **Extremely small concentration:**
magnitude of thermal velocities is much larger than the relative velocities induced by shear
 - **Isotropy in the angular distribution of collisions**
 - **Molecular chaos:**
no correlations in the velocities or positions of colliding particles
 - **Absence of friction between particles and walls:**
silos can not be modeled with kinetic theory
- 
- Binary collisions**

Kinetic Theory – Range of applicability



[Forterre and Pouliquen, Ann. Rev. Fluid Mech. (2008)]

Kinetic Theory – Extended theories

Jenkins, *Dense shearing flows of inelastic disks*. Phys.Fluids (2006)

Vescovi, Di Prisco & Berzi, *From solid to granular gases: the steady state for granular materials* (2013)

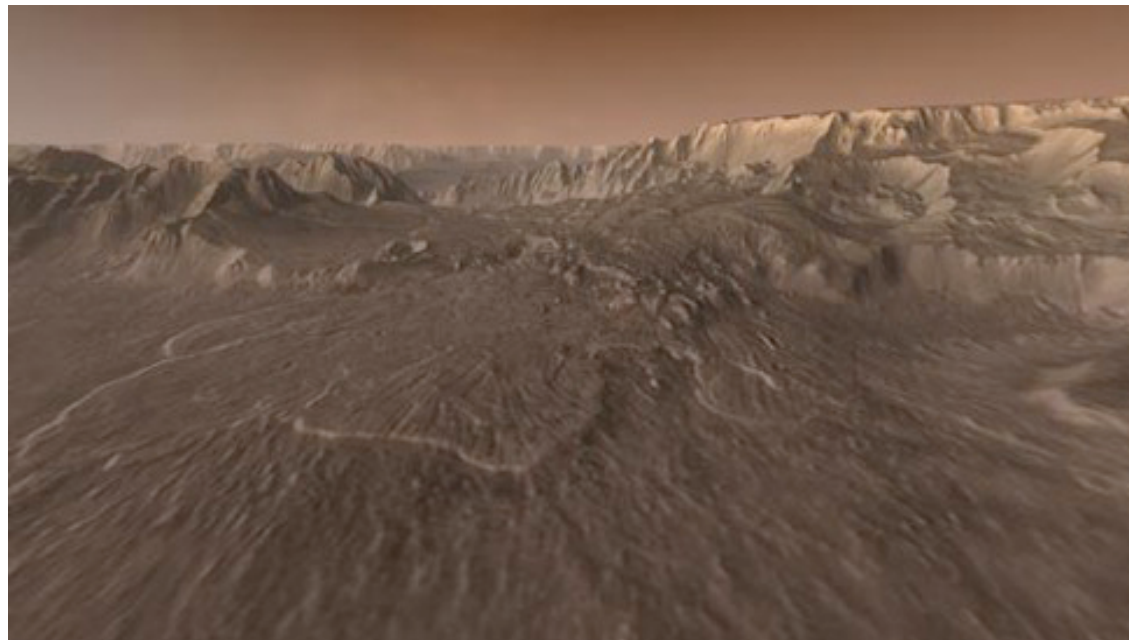
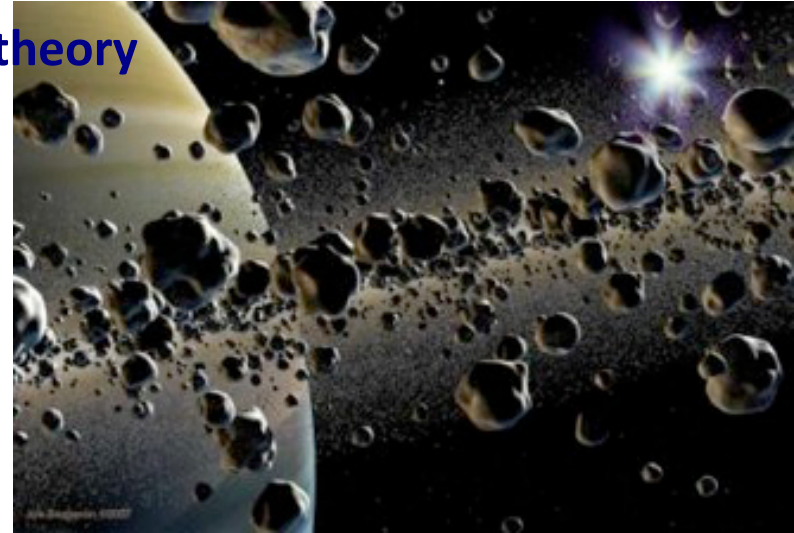
[...]

Granular material: continuum approach



Solid: soil mechanics

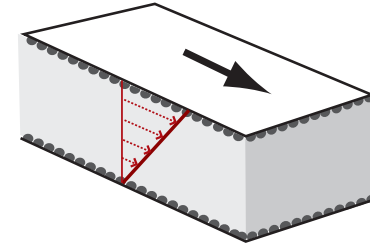
Gas: kinetic theory



Liquid ??

Dense (slow) flows
and inertial regime

Inertia number



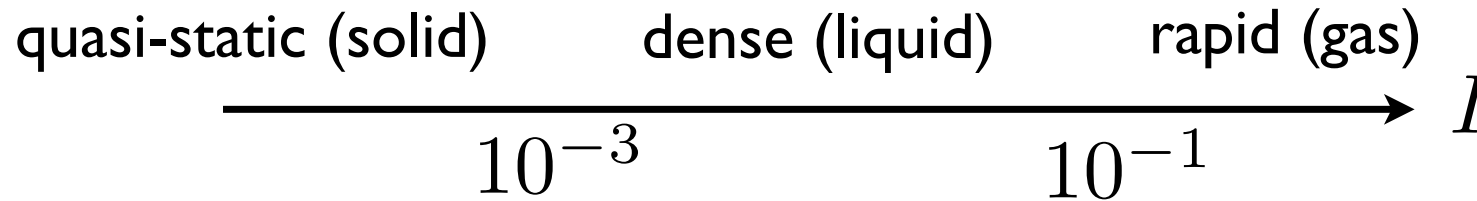
For **large** systems – and **rigid** grains

Only based on dimensional analysis

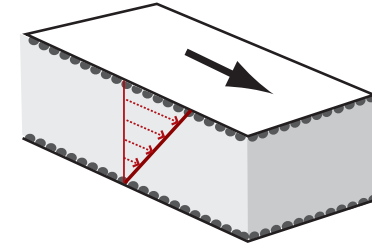
The transition can be described through a **single dimensionless number**

$$I = \frac{\dot{\gamma}d}{\sqrt{\frac{p}{\rho_p}}}$$

Inertia number



Inertia number



$$I = \frac{\dot{\gamma}d}{\sqrt{\frac{P}{\rho_p}}}$$

I = micro time scale / macro time scale

$$\frac{d}{\sqrt{\frac{P}{\rho_p}}}$$

microscopic time scale

time needed for a particle to fall in a hole of size d
under the pressure P

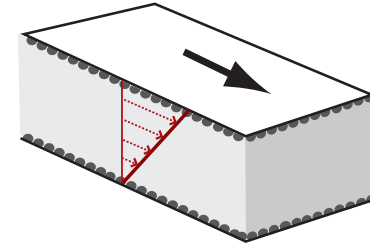
- typical time scale of rearrangements -

$$\frac{1}{\dot{\gamma}}$$

macroscopic time scale

linked to the mean deformation

Inertia number

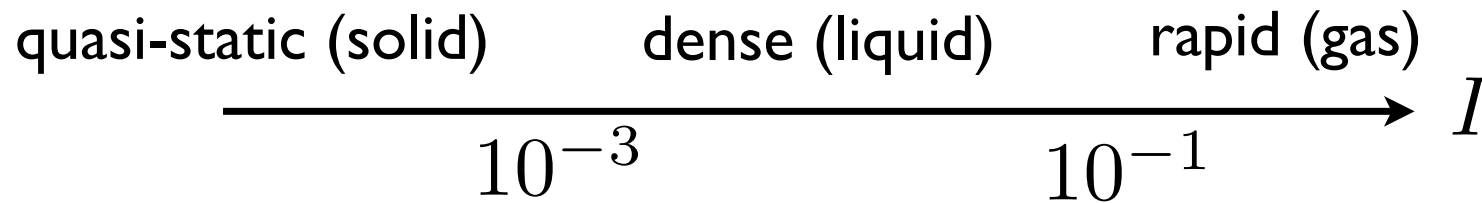


$$I = \frac{\dot{\gamma}d}{\sqrt{\frac{p}{\rho_p}}}$$

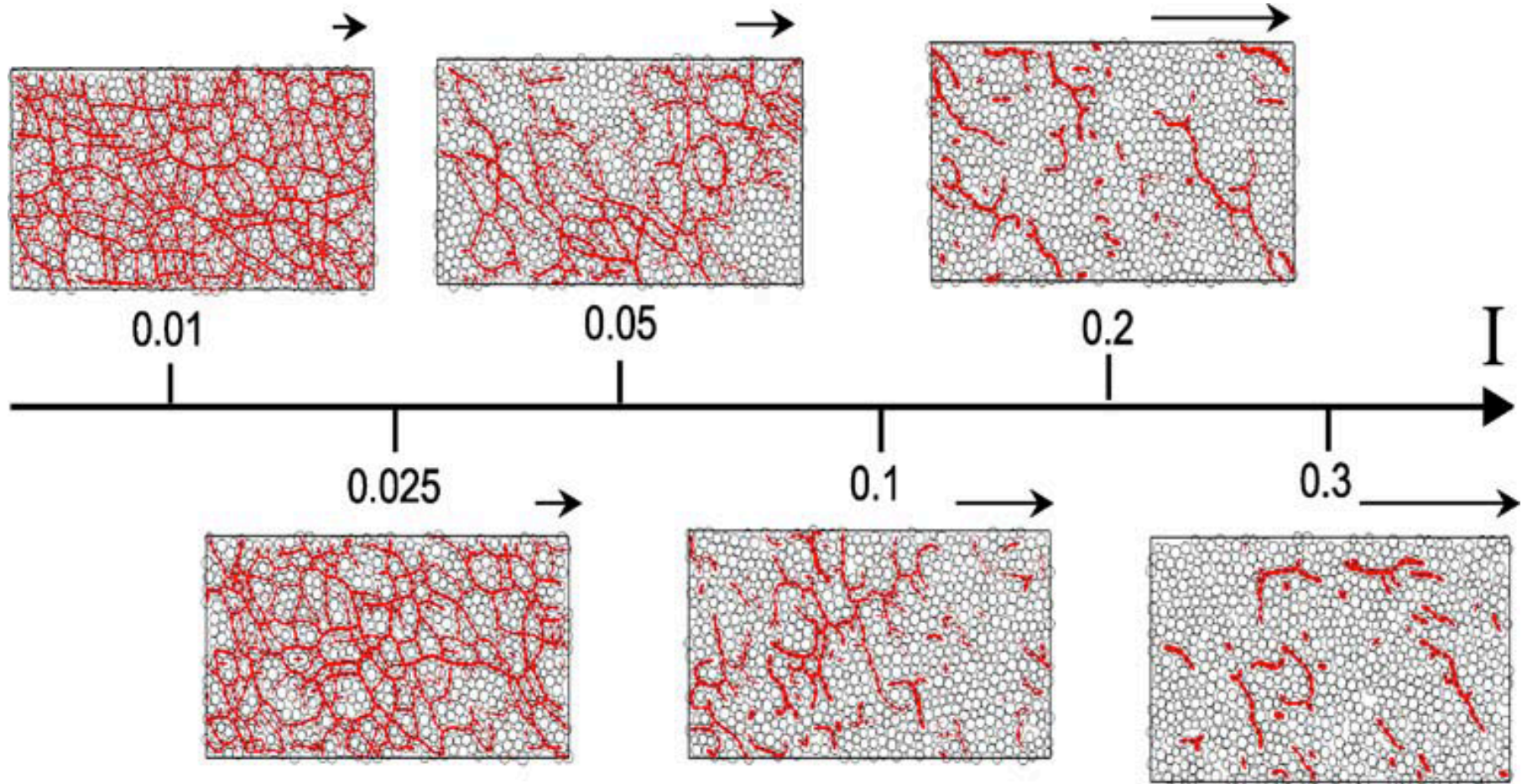
I = micro time scale / macro time scale

I **small** – quasi-static
macroscopic deformation is slow compared to
microscopic rearrangement

I **large** – rapid flows

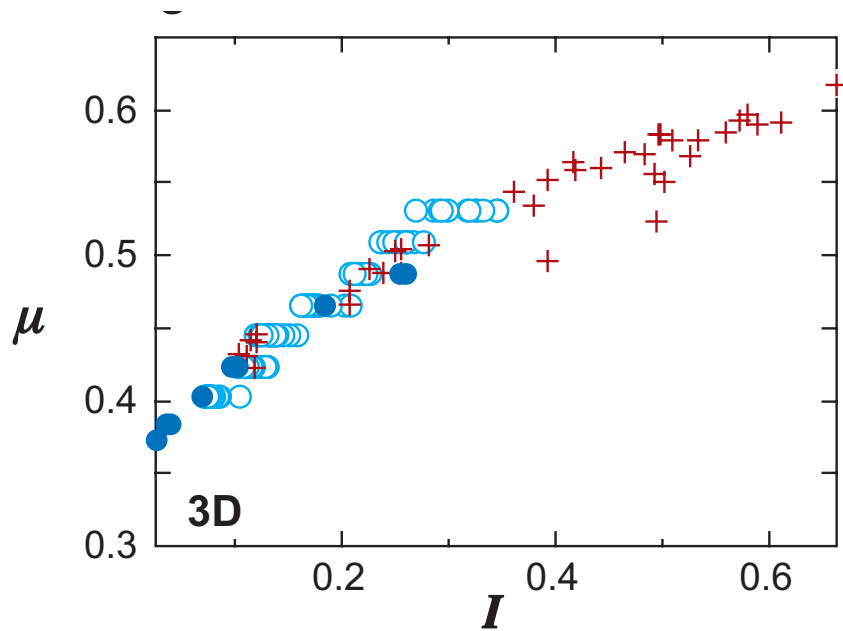


Quasistatic \rightarrow Dense \rightarrow Rapid



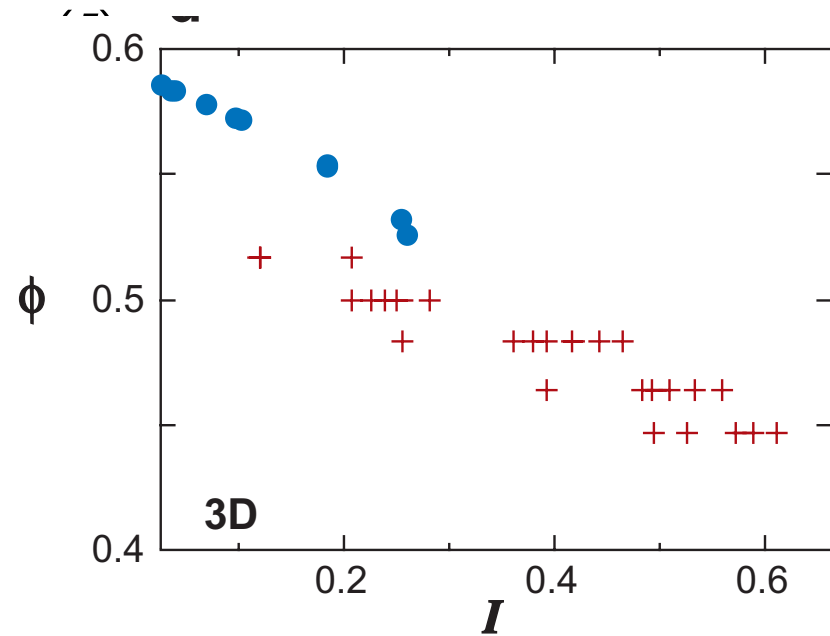
Pouliquen μ - I rheology

$$\tau = \sigma\mu(I) \quad \phi = \phi(I)$$



- inclined plane (exp, num)
- annular shear (exp)

(rigid grains)



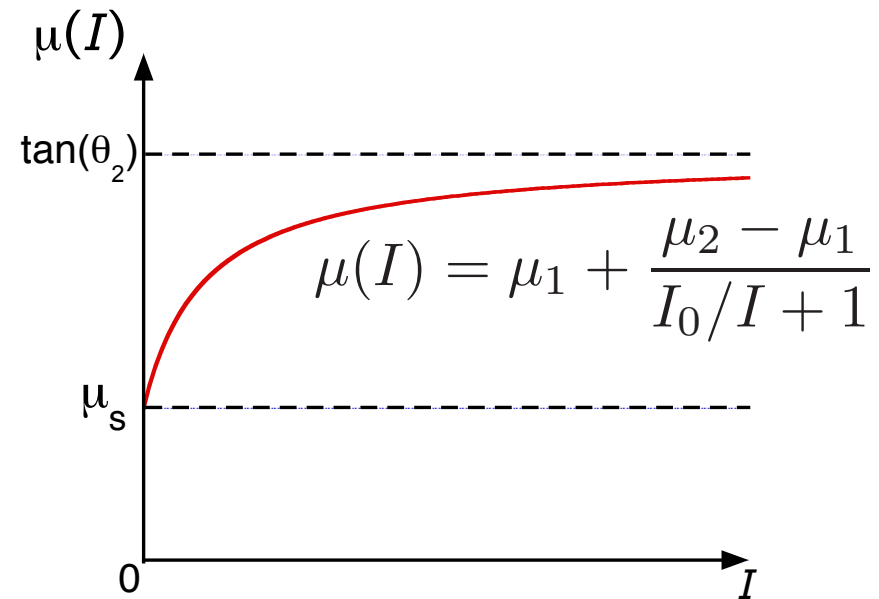
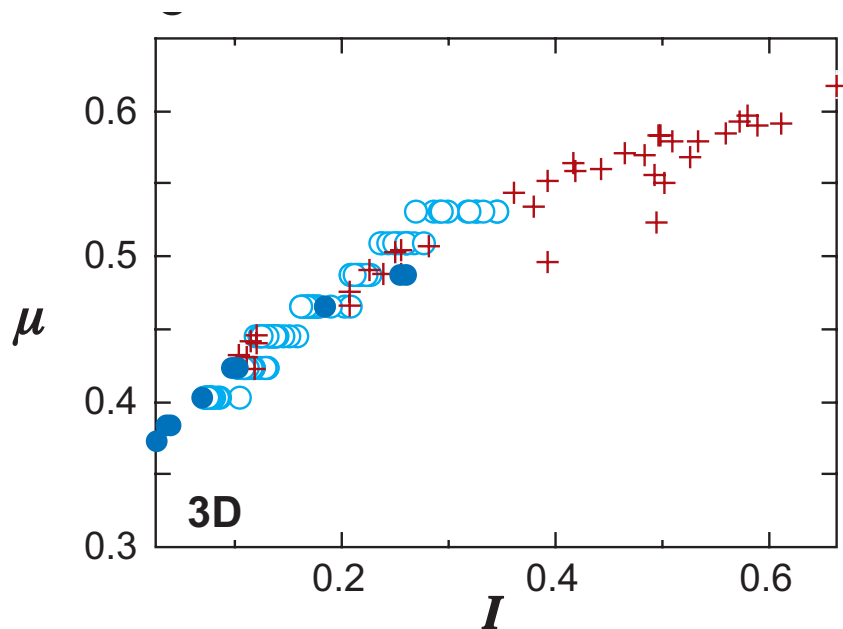
glass beads : $\mu_1 = \tan 21^\circ$
 $\mu_2 = \tan 33^\circ \quad I_0 = 0.3$

Pouliquen μ - I rheology – local constitutive relation

$$\tau = \sigma\mu(I)$$

$$\phi = \phi(I)$$

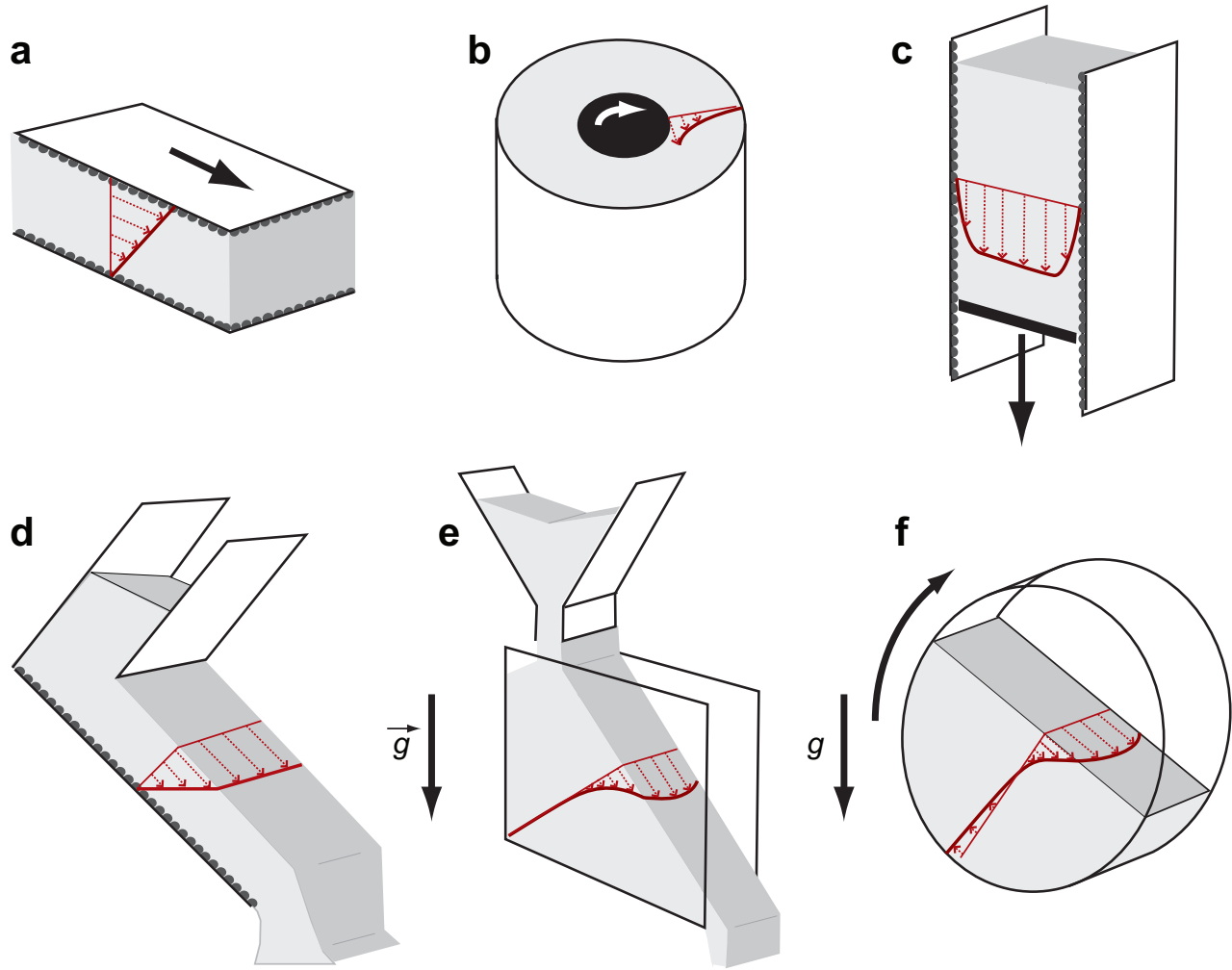
(rigid grains)



- inclined plane (exp, num)
- annular shear (exp)

glass beads : $\mu_1 = \tan 21^\circ$
 $\mu_2 = \tan 33^\circ$ $I_0 = 0.3$

Different geometries



[Forterre and Pouliquen, Ann. Rev. Fluid Mech. (2008), Weinhart et al. Phy. Fluids (2013)]

Pouliquen μ -I rheology – Tensorial extension

1) incompressible media, no normal stress difference

$$\sigma_{ij} = -P\delta_{ij} + \tau_{ij}$$

2) $\dot{\gamma}_{ij}$ et τ_{ij} are colinear

$$\tau_{ij} = \frac{\mu(I)P}{|\dot{\gamma}|} \dot{\gamma}_{ij}$$

effective viscosity

$$\dot{\gamma}_{ij} = \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i}$$

$$|\dot{\gamma}| = \sqrt{\frac{1}{2} \dot{\gamma}_{ij} \dot{\gamma}_{ij}}$$

(Drucker-Prager criterion)

Flow threshold $|\tau| = \mu_1 P$

$$|\tau| = \sqrt{\frac{1}{2} \tau_{ij} \tau_{ij}}$$

Pouliquen μ -I rheology – Tensorial extension

1) incompressible media, no normal stress difference

$$\sigma_{ij} = -P\delta_{ij} + \tau_{ij}$$

2) $\dot{\gamma}_{ij}$ et τ_{ij} are colinear

$$\tau_{ij} = \frac{\mu(I)P}{|\dot{\gamma}|} \dot{\gamma}_{ij}$$

effective viscosity

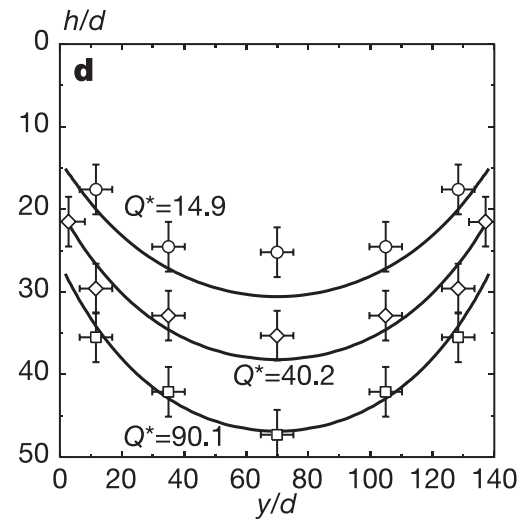
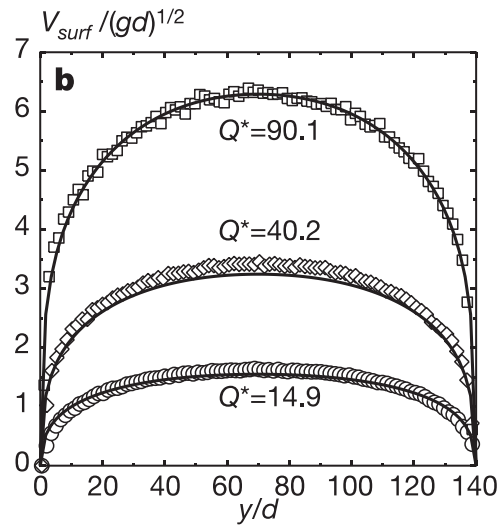
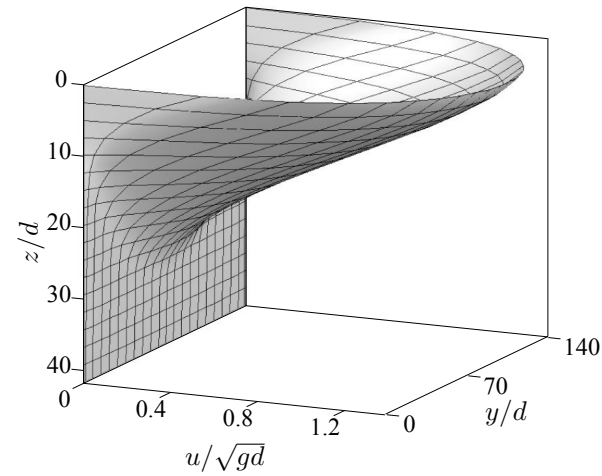
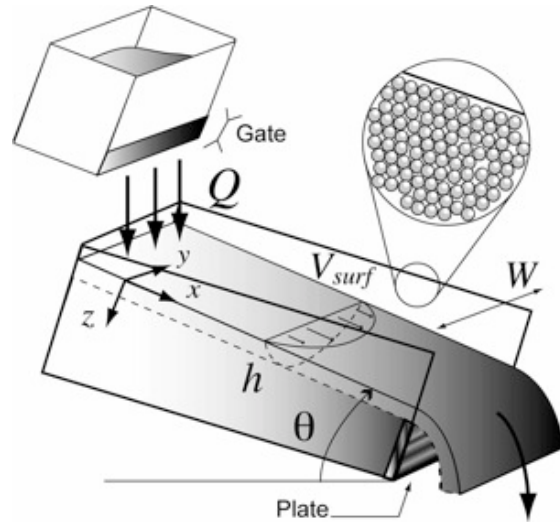
$$\dot{\gamma}_{ij} = \frac{\partial u_i}{\partial x_j} + \frac{\partial u_j}{\partial x_i}$$

$$|\dot{\gamma}| = \sqrt{\frac{1}{2} \dot{\gamma}_{ij} \dot{\gamma}_{ij}}$$

OK for: free surface flows between rough walls (Jop, 2006)
flows down an inclined plane (Forterre, 2006)

[Jop et al. Nature (2006)]

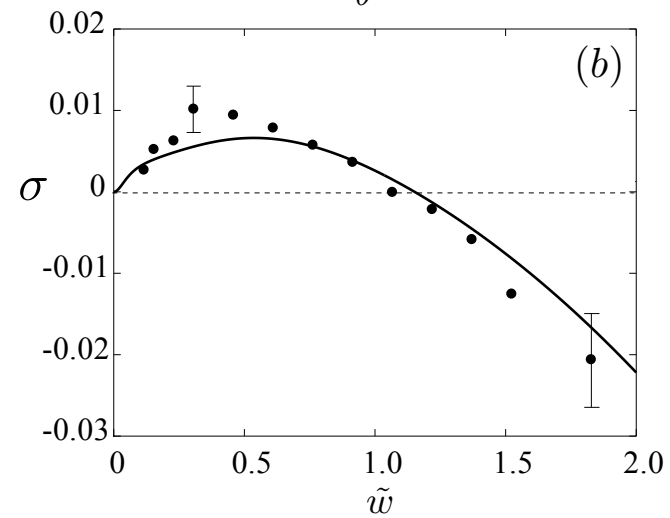
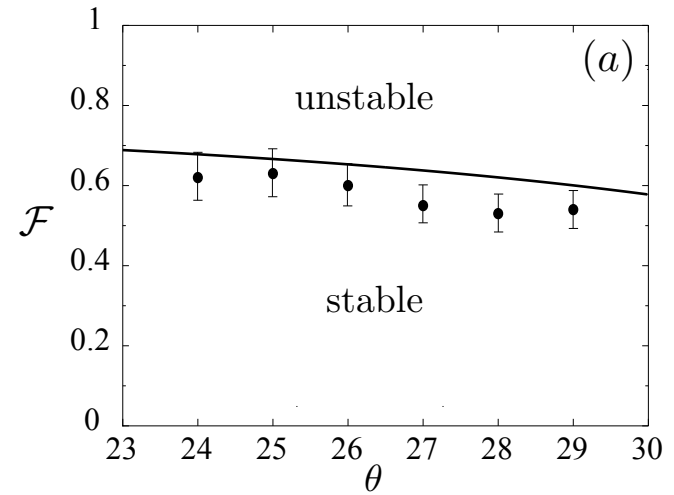
Test of the viscoplastic law : I- Heap flows



Jop et al. Nature 2006

Test of the viscoplastic law :

2 - Surface instability

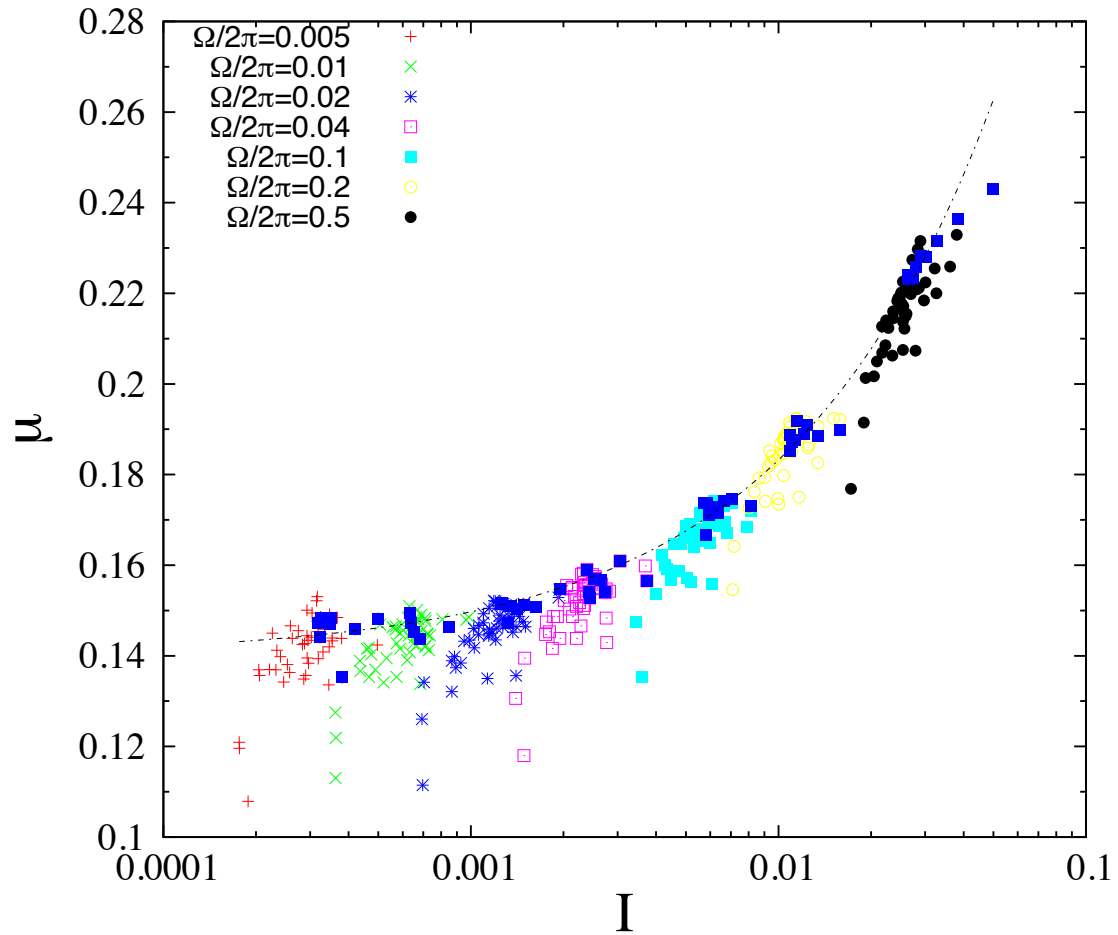


Forterre J. Fluid Mech. 2006

Limits of μ -I rheology

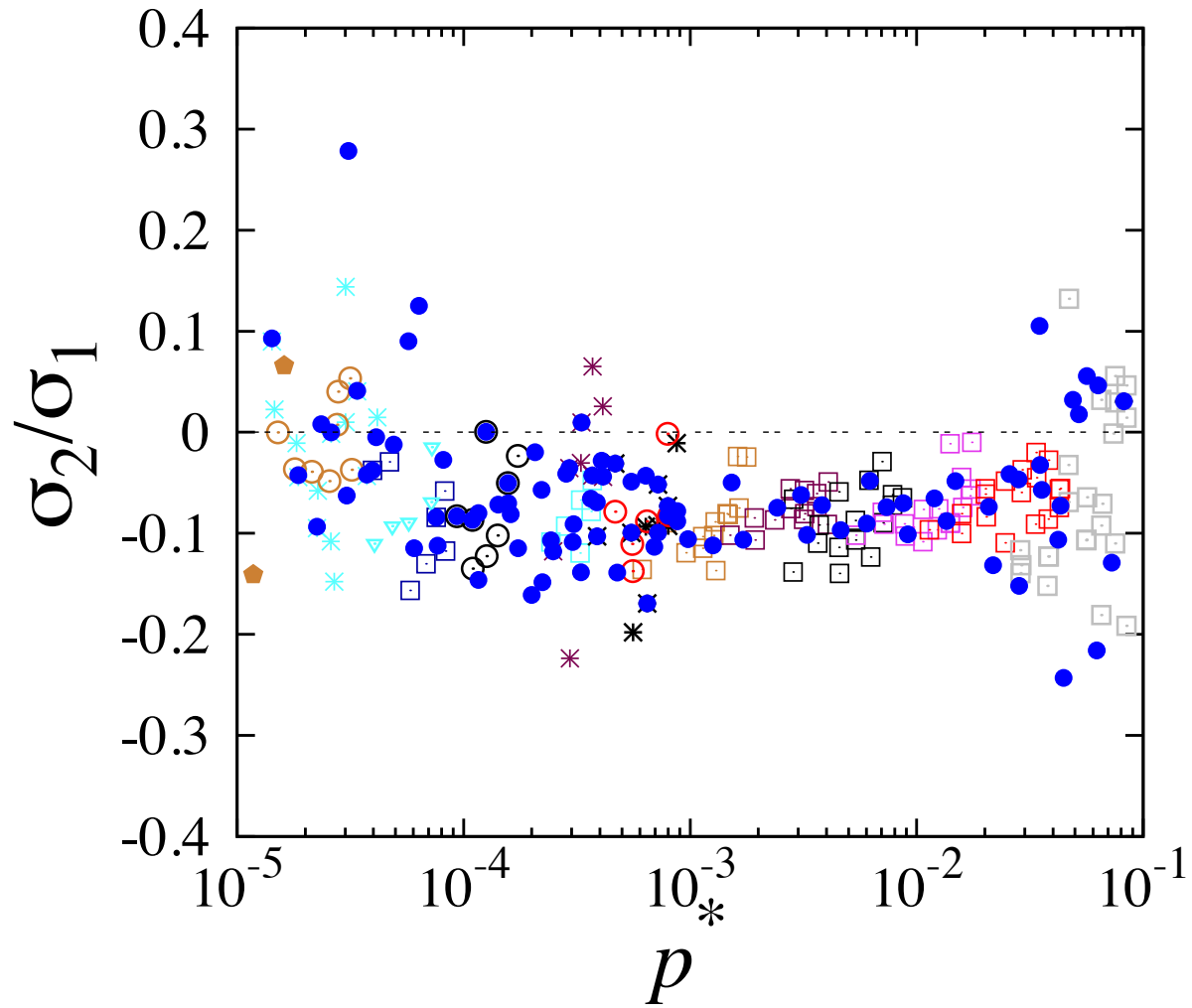
- **Microscopic origin**
phenomenological – some work in [Weinhart et al. Phy. Fluids (2013)]
- **Transition to Quasi-static regime**
shear bands not described
Transition between rate-independent and rate-dependent regimes
1 solution: **Non-local models**
- **Transition to Kinetic regime**
collisional flows not correctly described
modified kinetic theory by introducing a rate-independent term

Non local effects



[Singh et al., under review New J. Phys. (2015)]

Solid - fluid duality



[Singh et al., under review New J. Phys. (2015), Weinhart et al. Phys. Fluids (2013)]